

US Army Corps of Engineers Alaska District Soils and Geology Section



GEOTECHNICAL REPORT

MERTARVIK TOWNSITE Newtok, Alaska



MEMORANDUM FOR CEPOA-PM-C

SUBJECT: Geotechnical Report, Mertarvik Townsite - Newtok, Alaska

- 1. Enclosed are five bound copies of the Geotechnical Report for the subject project. Included with the report are the Project Location and Vicinity Map, Explorations Location Map, Exploration logs, lab testing results and design recommendations for the development of the road and evacuation center.
- 2. Questions should be addressed to Gregory Carpenter at 753-2684 or to Chuck Wilson at 753-2687.

Encl.

JAMES W. PEKAR, PE Chief, Geotechnical Services

CONCUR:

Carpenter 1

GEOTECHNICAL REPORT MERTARVIK TOWNSITE NEWTOK, ALASKA

FEBRUARY 2008

1. INTRODUCTION

The Alaskan Native Village of Newtok is located on the banks of the Ninglick and Kealavik Rivers, about 90 miles northwest of Bethel, in the Yukon-Kuskokwim Delta Region. The continued existence of the village at its present location is being threatened by advancing erosion caused by the Ninglick River which connects the Bering Sea with Baird Inlet. After years of erosion study, the Newtok Traditional Council concluded in 1994 that relocation of the entire village was the best solution, and has since been pursuing this action. The selected relocation site is known as Mertarvik shown in Photograph 1. This relocation requires an understanding of the subsurface conditions of the new site. Therefore, a geotechnical exploration was planned by the US Army Corps of Engineers - Alaska District (USACE-AD) for the summer of 2007.

The results of that exploration are presented in this report. The purpose of the investigation was to identify general surface and subsurface conditions pertinent to the design and development of the new community. In particular, it was to develop foundation recommendations and alternatives for an Evacuation Center and general considerations and recommendations for a Village Access Road. The exploration consisted of drilling test borings for proposed components of the townsite. This report presents the recommendations and alternatives and the results of the exploration and the laboratory testing program, as well as general site observations.

2. PROJECT DESCRIPTION AND LOCATION

This project consists of constructing a road from tidewater to a proposed village relocation site and an Evacuation Center that will be used if the existing village of Newtok is again flooded. In addition to the items mentioned above, the exploration also gathered preliminary information for other components of the relocation including a barge landing lay-down area, a village well, a landfill and sewage disposal area, and the general area of the village relocation site.

The proposed site is located on the north side of Nelson Island in western Alaska. The site is approximately 12 miles directly south of the village of Newtok, Alaska. A Project Location and Vicinity Map are enclosed as Figure 1.



Photograph 1. Drilling operation at Test Boring AP-21 showing vegetation and topography of Mertarvik with the Ninglick River in the background.

3. FIELD EXPLORATION

The subsurface exploration for the project was conducted from 6 through 15 September 2007. A total of 24 test borings were drilled to depths from about seven to 31.5 feet. These borings have been designated as AP-01 through AP-24. The exploration generally consisted of drilling and sampling from the ground surface to a predetermined depth depending on the boring location. However, several boring were terminated prior to reaching the predetermined depth due to auger refusal on rock. These borings were drilled with a track-mounted Mobile B-61 drill rig. The borings were advanced using 8-inch diameter hollow-stem auger. Denali Drilling, Inc. of Anchorage supplied the drilling equipment, landing craft used to move the equipment to the site, and personnel to perform the exploration. An engineer with the USACE-AD supervised the operation and logged the test borings. Field classification of the soils is in accordance with ASTM D 2488, "Standard Practice for Description

and Identification of Soils (Visual - Manual Procedure)."

The test borings were located in the field with a handheld GPS unit using predetermined boring location coordinates and are only as accurate as the method implies. The elevations of the borings were estimated by correlating horizontal coordinates to topographical information obtained by aerial photography. The approximate locations of the test borings and conceptual plans are shown on the Exploration Location Map enclosed as Figure 2.

Samples were collected at either two or five feet below the ground surface and 5-foot intervals thereafter. Soil samples were procured with a 2.5-inch inside diameter split spoon sampler driven with a 340-pound hammer falling 30 inches using a rope and cathead to lift the hammer. During drive sampling, the split spoon sampler was advanced 18 inches ahead of the auger or to driving refusal. The number of blows required to drive each 6-inch increment is recorded on the exploration logs. The blow count is an indication of the relative density or consistency of the soil although in the areas where permafrost was encountered the blow counts only indicate the hardness of the frozen soil.

4. LABORATORY TESTING AND SOILS CLASSIFICATION

A laboratory testing program was established to classify the soils encountered. These tests were performed in accordance with the current version of the following test methods:

- ASTM D 422, "Standard Test Method for Particle size Analysis of Soils".
- ASTM D 2216, "Standard Test Method for Laboratory Determination of Water (Moisture) Content of Soil and Rock by Mass".
- ASTM D 2487, "Standard Practice for Classification of Soils for Engineering Purposes (Unified Soil Classification System)".
- ASTM D 4318, "Test Method for Liquid Limit, Plastic Limit, and Plasticity Index of Soils".

The soil descriptions and classifications contained in this report and presented on the final exploration logs are the project engineer's interpretation of the field logs and results of the laboratory testing program. The stratification lines represent approximate boundaries between soil types; the transitions are often gradual or not discernible by drill action. The exploration logs are enclosed as Appendix A and the lab testing summary and grain size distribution curves are enclosed as Appendix B.

5. Regional Geology

Nelson Island is located in the lowlands between the mouth of the Yukon River

and the mouth of the Kuskokwim River. The island was formed by basalt flows from volcanic activity in the area and rises from the surrounding alluvial plane to an elevation of about 1485 feet. On the west coast of the island there are rock exposures that are several hundred feet high. In general the soils on the island are a product of the weathering of the basalt as exemplified in Photograph 2.



Photograph 2. Sample of material that is typical of the soil composed of residual basalt.

6. Site Conditions

<u>Surface</u>: The project is located on the north shoreline of Nelson Island and is bounded on the north by the Ninglick River which drains Baird Inlet. The barge landing and beginning of the road alignment is identified on the Location and Vicinity Map and can be related to the Test Boring Location Map. The relocation site is located on a hillside that slopes up to the south from sea level at the shoreline to an elevation of about 320 feet. The slope is relatively constant and generally is at about 15 percent.

The vegetation generally consists of tundra plants with areas along the drainages that have willows to eight-feet in height. The willows do not grow more than half way up the slope. Rock exposures are present at one location on the shoreline and there are large rock fragments on the surface at many locations on the site.

<u>Subsurface</u>: The subsurface conditions vary across the site. The thickness of the soil over bedrock was encountered from as little as four feet to more than 31.5 feet. The permafrost conditions across the site are similarly variable. The table below summarizes the conditions encountered at each of the boring locations.

TABLE 1 SUMMARY OF BORING DATA

BORING	DEPTH OF BORING (ft)	PEAT THICKNESS	DEPTH TO ROCK (ft)	DEPTH TO PERMAFROST
	DOMING (IL)	(ft)	ROCK (It)	(ft)
AP-01	15.8	1.5	13.0	not encountered
AP-02	15.0	2	12	not encountered
AP-03	16.0	1.5	not encountered	2.0
AP-04	11.5	1.5	8.5	not encountered
AP-05	16.5	1.5	not encountered	2.0
AP-06	16.5	1.5	not encountered	2.0
AP-07	9.0	1.0	9.0	3.5
AP-08	16.5	1.5	not encountered	not encountered
AP-09	16.5	1.5	not encountered	not encountered
AP-10	9.0	1.5	9.0	not encountered
AP-11	16.5	1.0	not encountered	1.0
AP-12	15.8	1.0	not encountered	1.5
AP-13	11.0	1.0	11.0	2.0
AP-14	14.0	1.0	14.0	2.5
AP-15	22.0	2.0	13.0	3.0
AP-16	20.1	2.0	16.5	2.0
AP-17	12.0	1.5	9.0	2.5
AP-18	10.3	2.0	10.0	2.0
AP-19	10.2	1.5	9.0	not encountered
AP-20	7.0	1.5	4.0	not encountered
AP-21	21.5	2.0	12	not encountered
AP-22	22.0	1.0	21.0	not encountered
AP-23	31.5	1.5	not encountered	2.5
AP-24	13.5	1.5	13.5	3.0

In general, the soils above bedrock are relatively uniform and are a product of the weathering of the underlying bedrock. On the surface there is a peat layer that is generally about 18 inches in thickness but varies from one to two feet thick. Beneath the peat there is a layer of silt with organics that are a product of roots. The amount of organics varies and tends to decrease with depth. As the bedrock surface is approached, rock fragments become more prevalent in the residual soil. Finally, the rock surface is encountered. The rock becomes more competent with depth. Most of the soils are frost susceptible and have a frost classification of F4 although a few of the soil samples near the bedrock surface have a frost classification of F2. The soils are generally wet in thawed areas and at many locations water ponded around the tracks on the drill rig while drilling. In frozen areas the soil contains ice as small crystals to layers of ice several inches thick. From the observations during drilling, it appears that the seasonal thaw depth is about two feet due to the insulation provided by the tundra vegetation.



Photograph 3. Typical sample showing the ice content present in many of the frozen soils.



Photograph 4. Ice typical of many areas in the permafrost.

The permafrost conditions on the site vary greatly between boring locations. It appears that the permafrost has degraded at locations near drainage paths and in areas where water may pond. In general, the permafrost is either present within two to three feet of the ground surface or it is degraded to below the bedrock surface. One observation that was made at the site was that in some areas where permafrost degradation has occurred the subsidence of the ground surface was on the order of a couple of feet or more as shown in Photograph 5. For a detailed description of the subsurface conditions encountered at each boring location, see the exploration logs contained in Appendix A.

7. Analysis

The site is suitable for construction of the proposed road from the beach to the townsite and for the construction of the proposed evacuation center. Issues that require consideration and analysis for this project are the permafrost along the road alignment as well as the drainage along the proposed road



Photograph 5. One of many thaw features present on the site. Note the subsidence that has occurred as a result of thaw consolidation.

section. Another issue is the foundation type for support of the evacuation center. These issues are discussed below:

Roadway Design: The road from the barge landing to the proposed townsite will traverse various soil and thermal conditions. In general it would be appropriate to design the road as though the alignment was entirely on permafrost. It would be prudent to construct the road over the existing tundra with as little surface disturbance as practical. This may require the construction to be performed in the early spring when the ground remains frozen. The most important aspects of the road design will be the protection of the permafrost and drainage of water away from the road section. This must be considered for the structural section, construction technique and could be achieved by using a layer of insulation in the road section as well as a geotextile to reduce the impact of thaw settlements. The use of culverts at all drainage areas as well as at any location where ponding might occur should help remove water from the roadway section.

Evacuation Center Foundation: There are several options for the support of the proposed Evacuation Center. The foundation system with the least risk is a pile foundation installed with the pile tips on or embedded into the bedrock. Of special concern with a pile foundation is the elimination of frost jacking of the piles. A second option, with slightly more risk of movement due to frost action would be to construct a building pad of gravel or rock and then supporting the building on a triodetic foundation system. The foundation system with the most risk would be to construct a building pad with gravel or rock and then to support the building on a conventional foundation system.

8. Engineering Recommendations

Recommendations regarding design and construction of the access road and the foundation for the Evacuation Center are presented in this section. These recommendations are based on results of the test boring data, the results of laboratory testing, experience, and engineering judgment.

Access Road: The proposed access road from the barge landing to the proposed townsite will cross areas of permafrost alternating with thawed areas. The final design will be prepared by the design team for this project. It is recommended that the road be designed and constructed as if it were all on permafrost. The road should be constructed over the existing tundra with as little disturbance of the surface as practical. This may require that the construction occur in the spring when the ground remains frozen. Some leveling of the road alignment will be necessary to allow a geotextile to be placed as a separator on the roadbed. The road fill should consist of gravel or a crushed rock product with less than 5 percent fines. A surface layer with more than 5 percent fines may be considered for the driving surface. It would be prudent to include a layer of insulation in the road section. This insulation layer should be placed as near the surface of the road as possible while keeping sufficient cover to protect it from damage when road maintenance is performed. The road should be shaped to rapidly drain and sufficient culverts should be placed under the road (at the tundra surface) to ensure that water does not pond adjacent to the roadway section.

Evacuation Center Foundation: A pile foundation is recommended for support of the proposed Evacuation Center. The piles should be driven to a minimum depth of 30 feet or to bedrock if it is deeper than 30 feet. This may require drilling and driving the piles into the bedrock. The piles should have a protective shoe to prevent damage during driving. The shoe should be flush outside and have as large an annulus as possible. One appropriate type of shoe is manufactured by Tubex. Piles installed to or into bedrock will have an allowable capacity of 20 kips on six-inch pipe, 35 kips on eight-inch pipe, and

55 kips on ten-inch pipe. The contractor should not be allowed to predrill any holes larger than the pile diameter minus one inch. The piles must be driven into a tight predrilled hole to create sufficient friction to resist the frost jacking forces on the pile.

A foundation alternative would be to construct a building pad of gravel or processed rock product that is a minimum of five feet thick and to support the structure on a triodetic foundation system. This alternative does have some risk of differential settlement if the building footprint is above both frozen and thawed ground. With this type of foundation system the differential settlement would result in little or no structural damage to the building but the building would become unlevel.

The final foundation alternative and the alternative with the most risk of differential settlement is to create a building pad as described above and then support the building on a conventional foundation. This alternative is not recommended unless it is shown that the soils are thawed above the bedrock surface under the entire building footprint.

Enclosures:

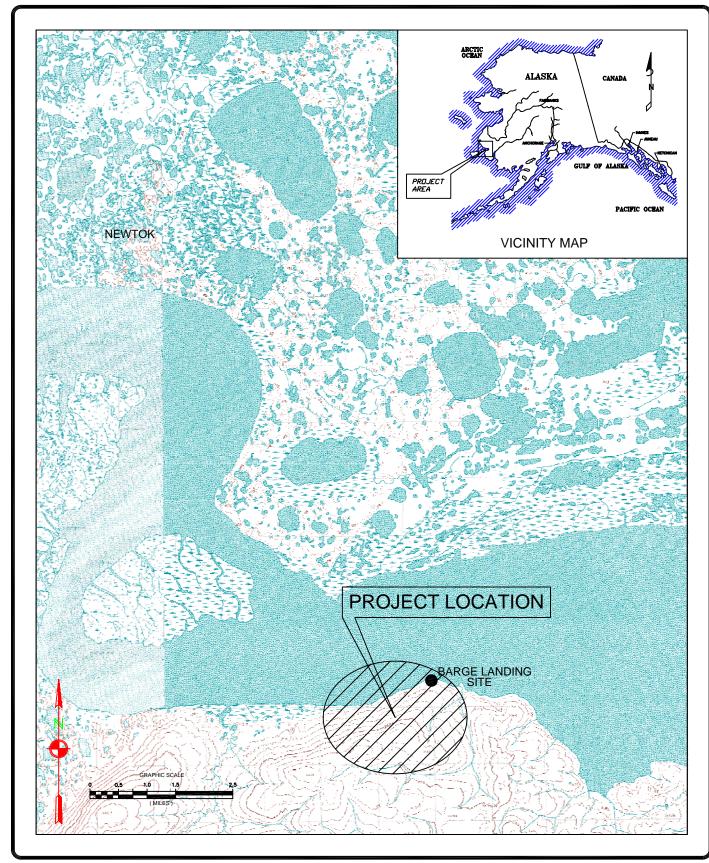
Figure 1 - Project Location and Vicinity Map

Figure 2 - Exploration Location Map

Appendix A - Exploration logs

Appendix B - Grain size distribution curves

CORPS OF ENGINEERS U.S. ARMY





ALASKA DISTRICT CORPS OF ENGINEERS SOILS AND GEOLOGY

LOCATION AND VICINITY MAP MERTARVIK TOWNSITE NEWTOK, ALASKA SCALE: GRAPHICAL

DATE: FEBRUARY 2008

DRAWN/RVW: IJR/GWC

FIGURE 1

CORPS OF ENGINEERS U.S. ARMY BARGE LANDING SITE DENOTES TEST BORING LOCATION



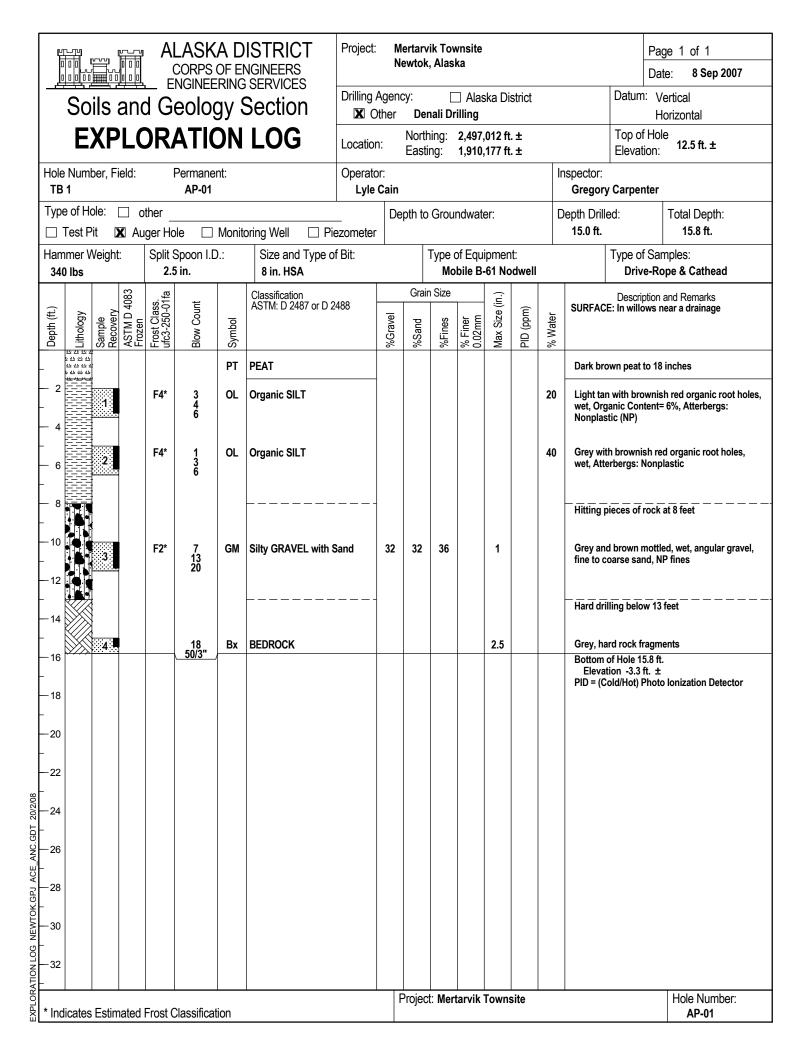
ALASKA DISTRICT CORPS OF ENGINEERS SOILS AND GEOLOGY

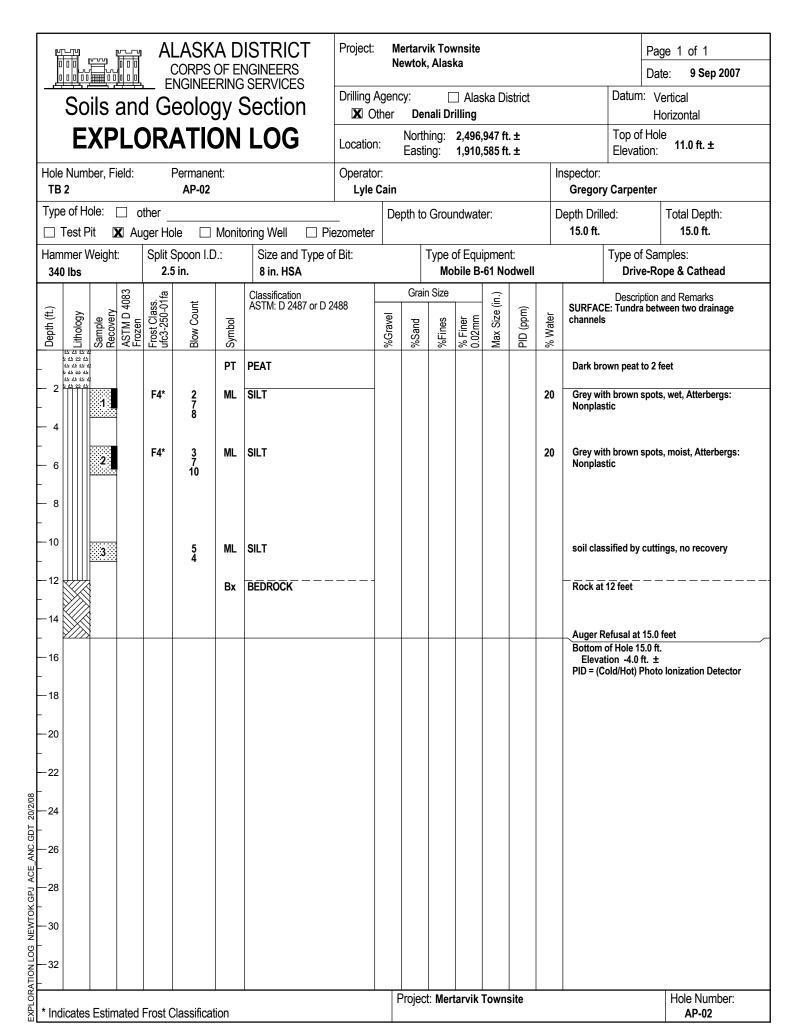
TEST BORING LOCATION MAP MERTARVIK TOWNSITE NEWTOK, ALASKA SCALE: NTS

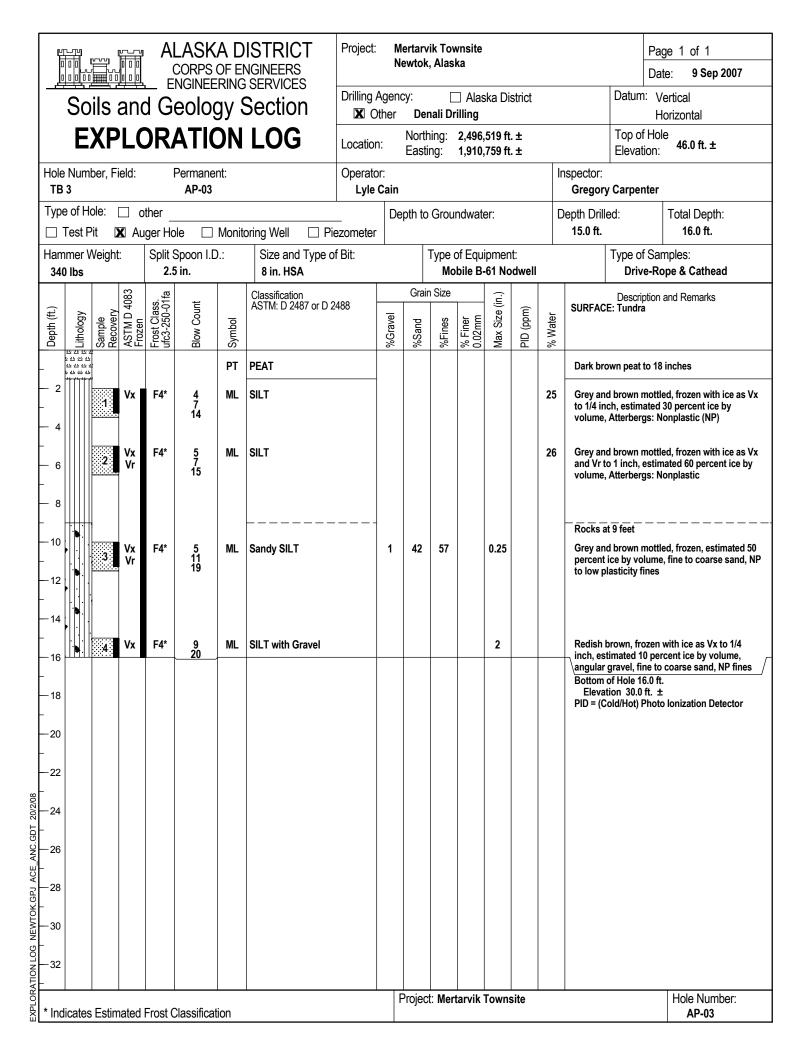
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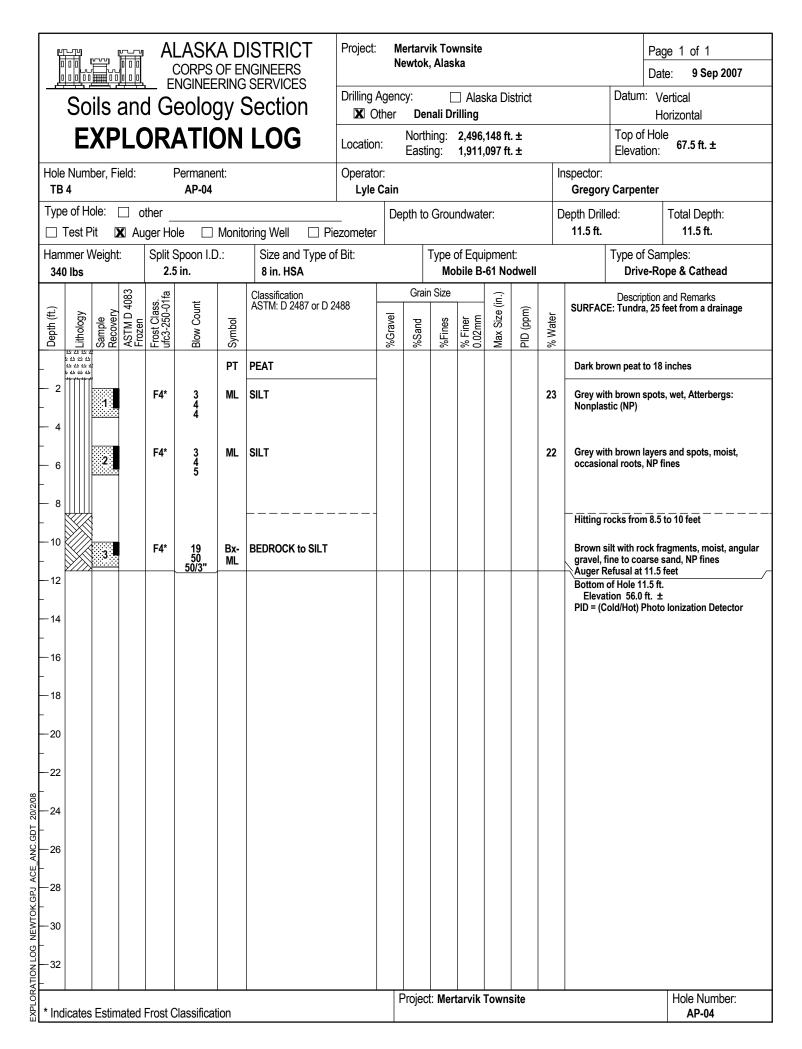
FIGURE 2

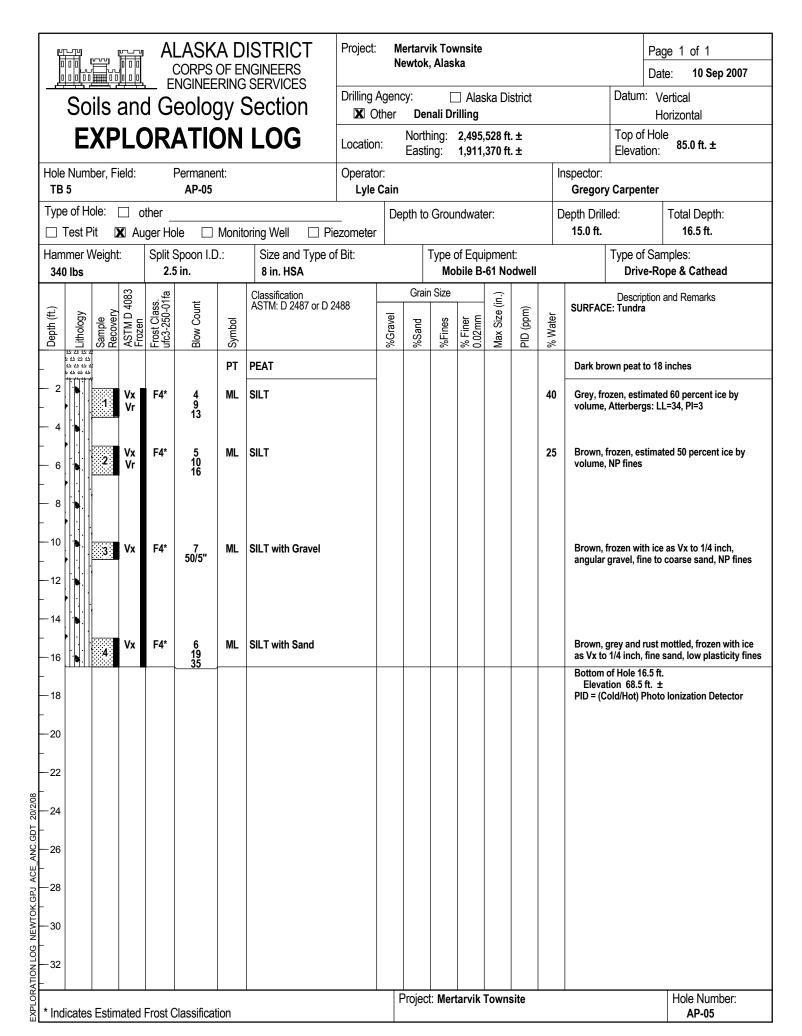
Appendix A Exploration Logs

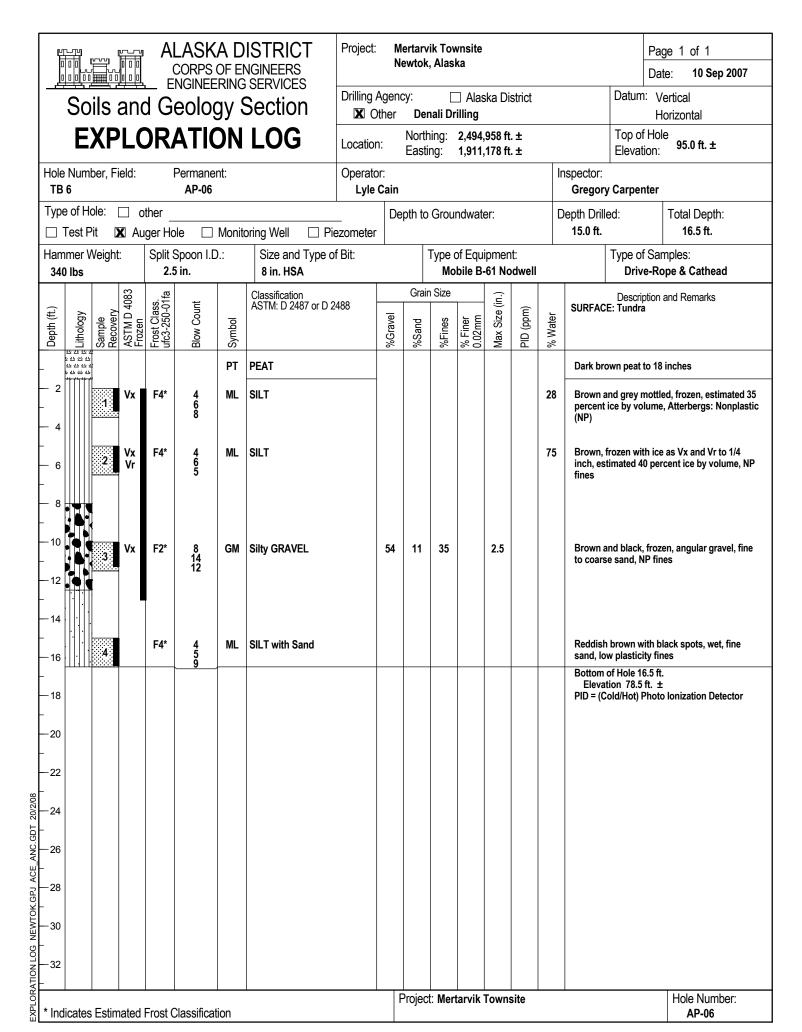


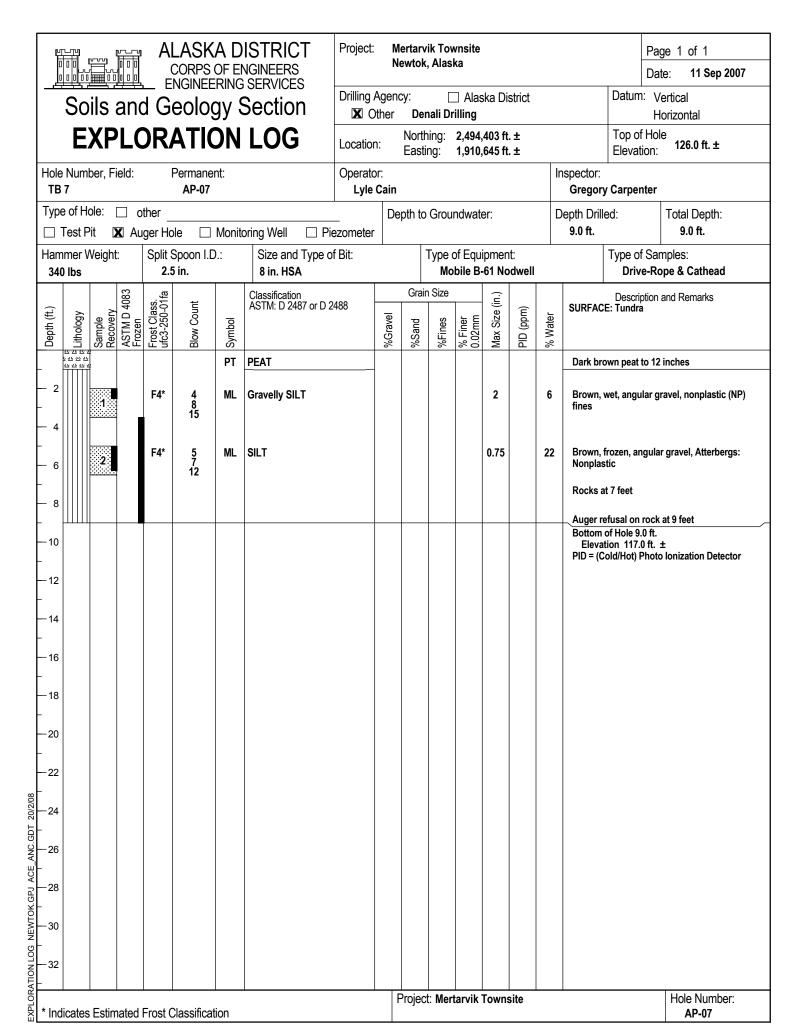


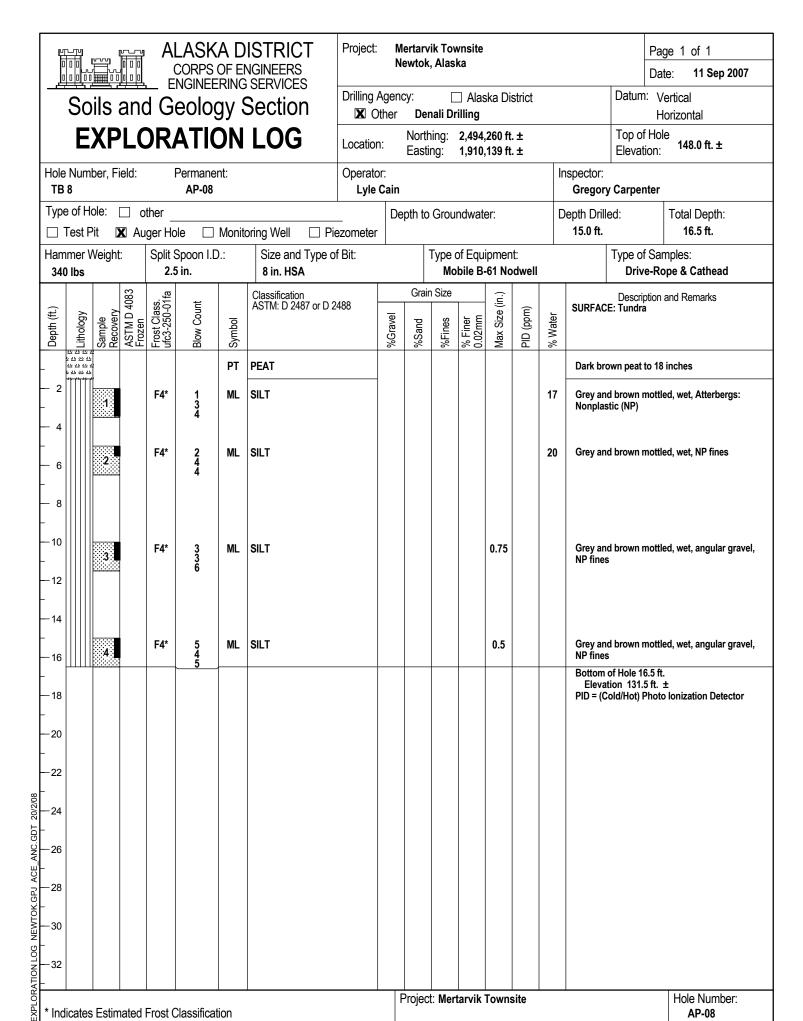






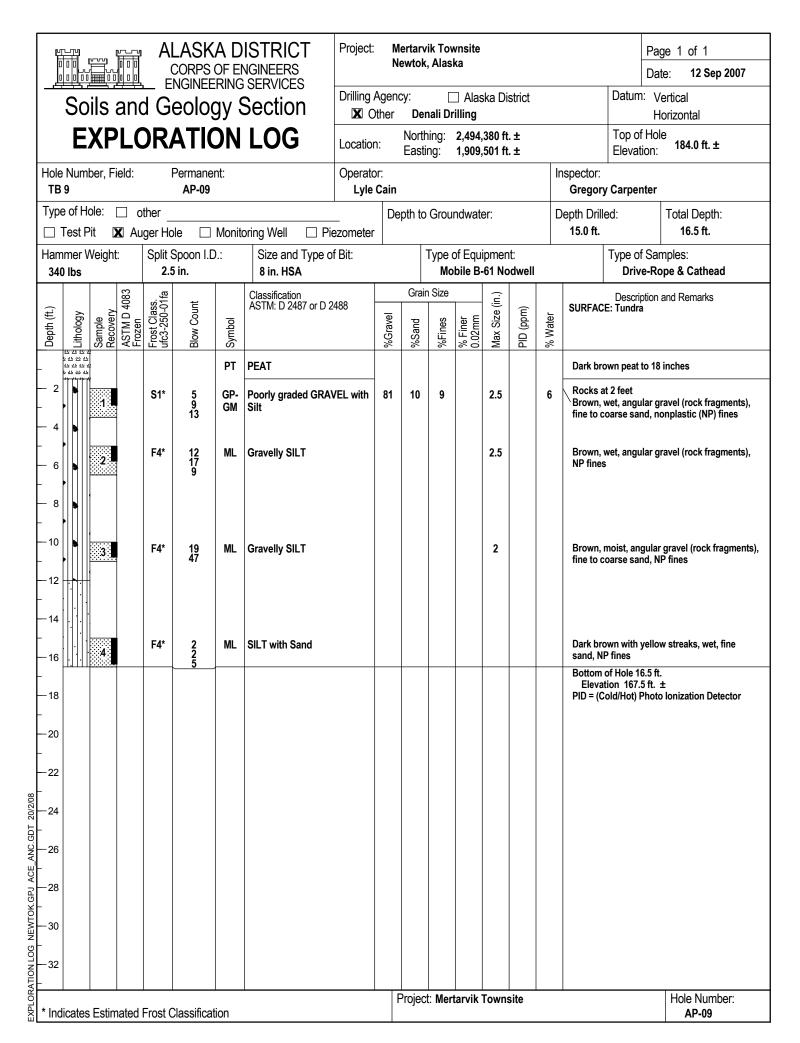


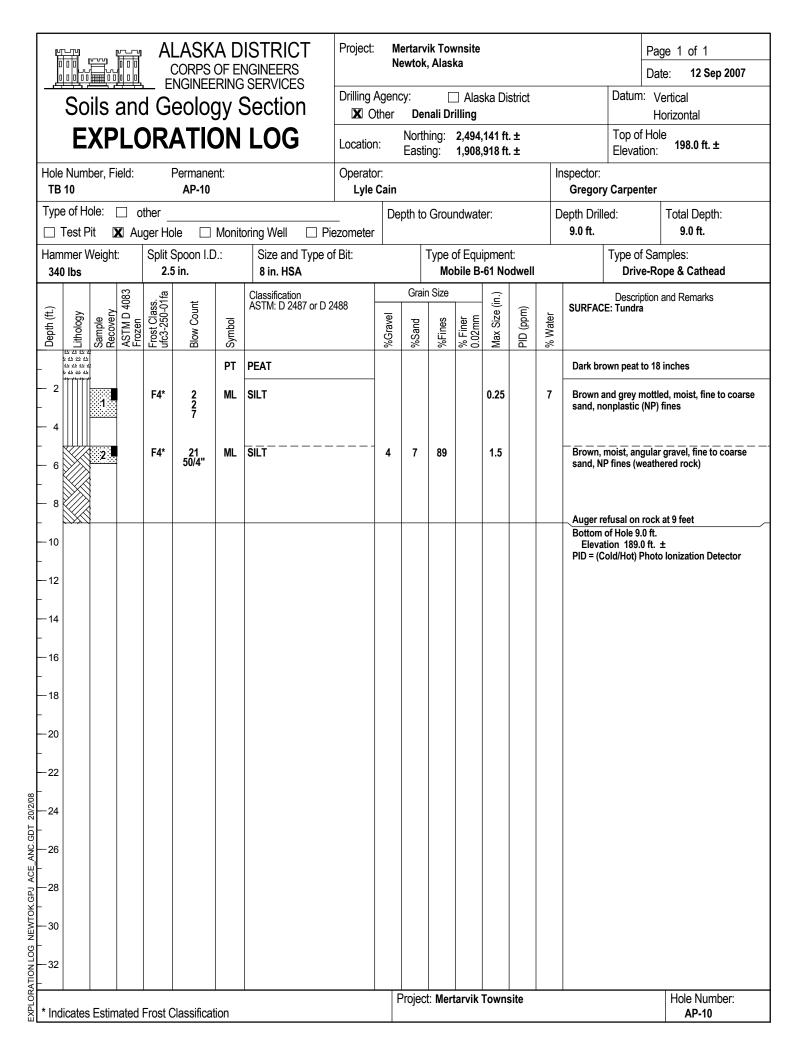


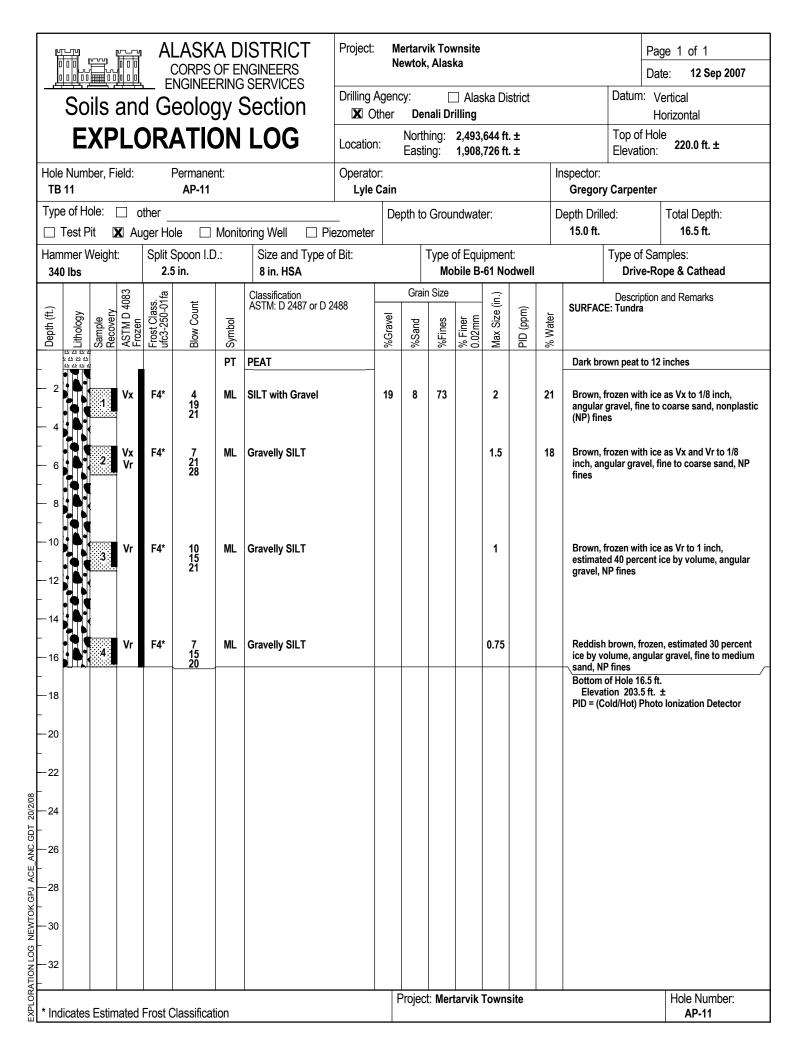


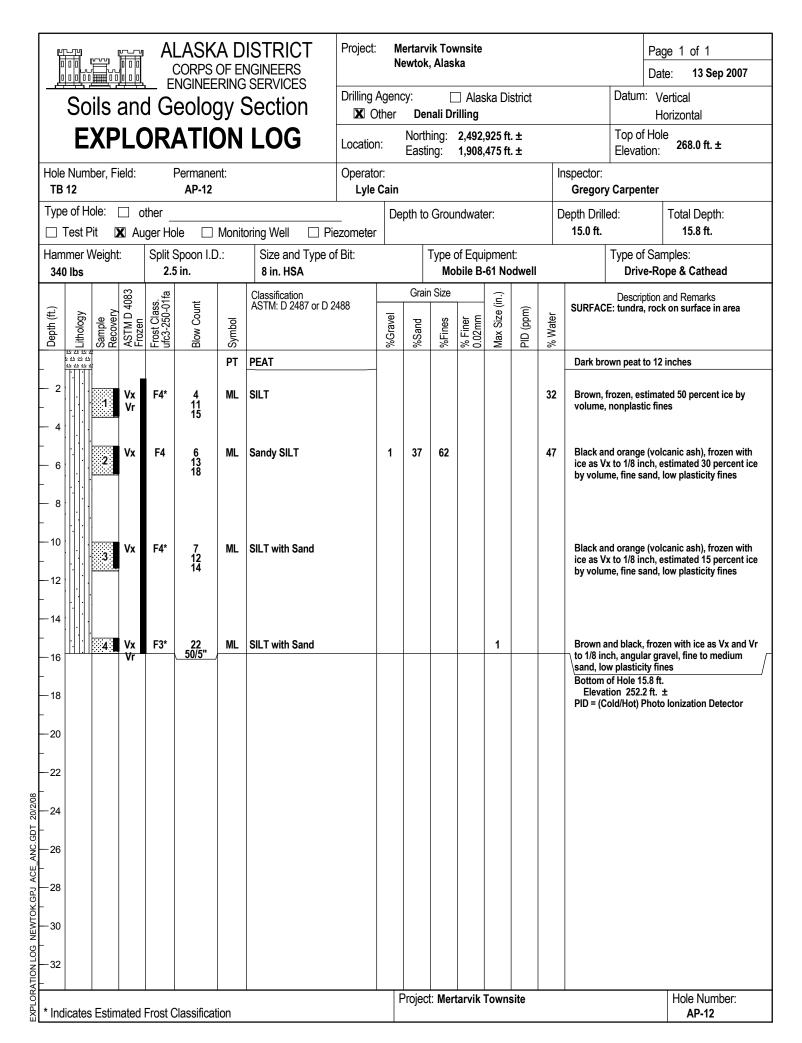
AP-08

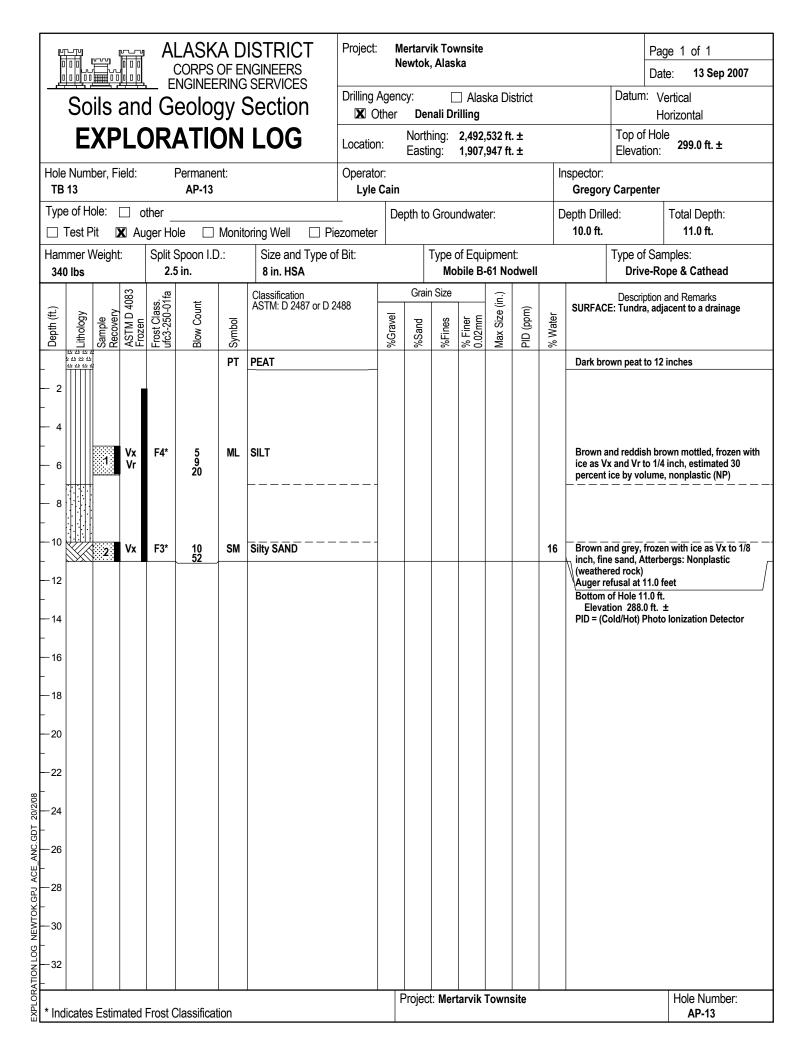
Indicates Estimated Frost Classification

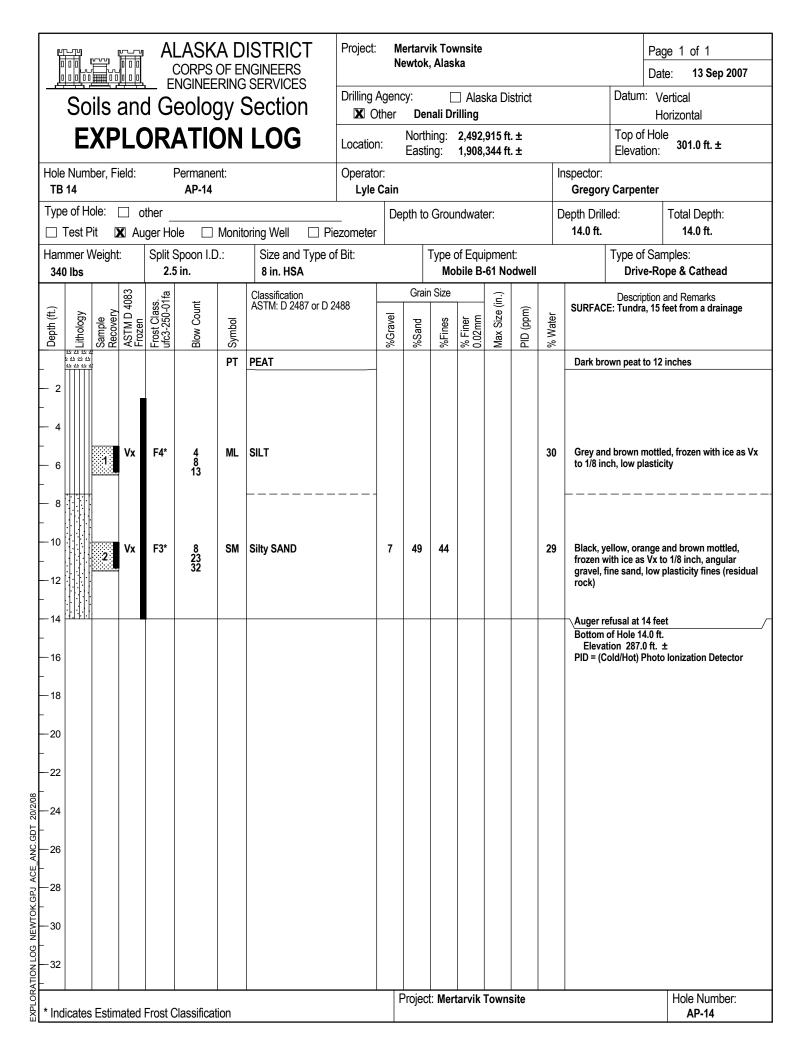


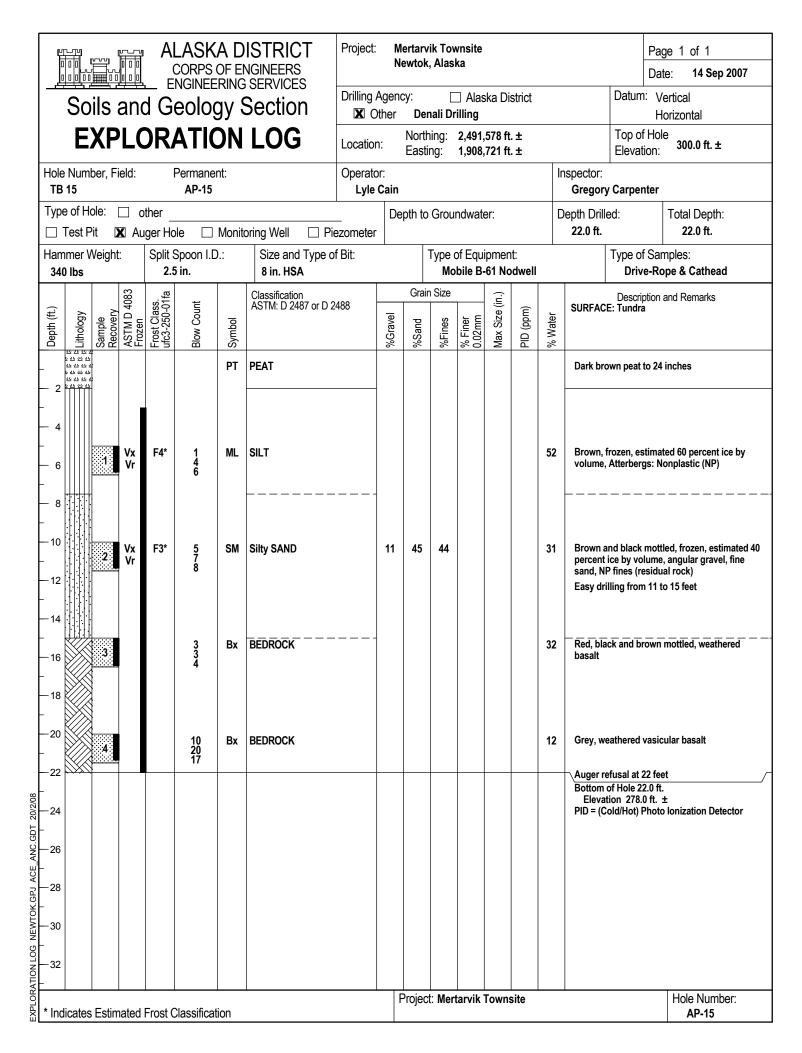


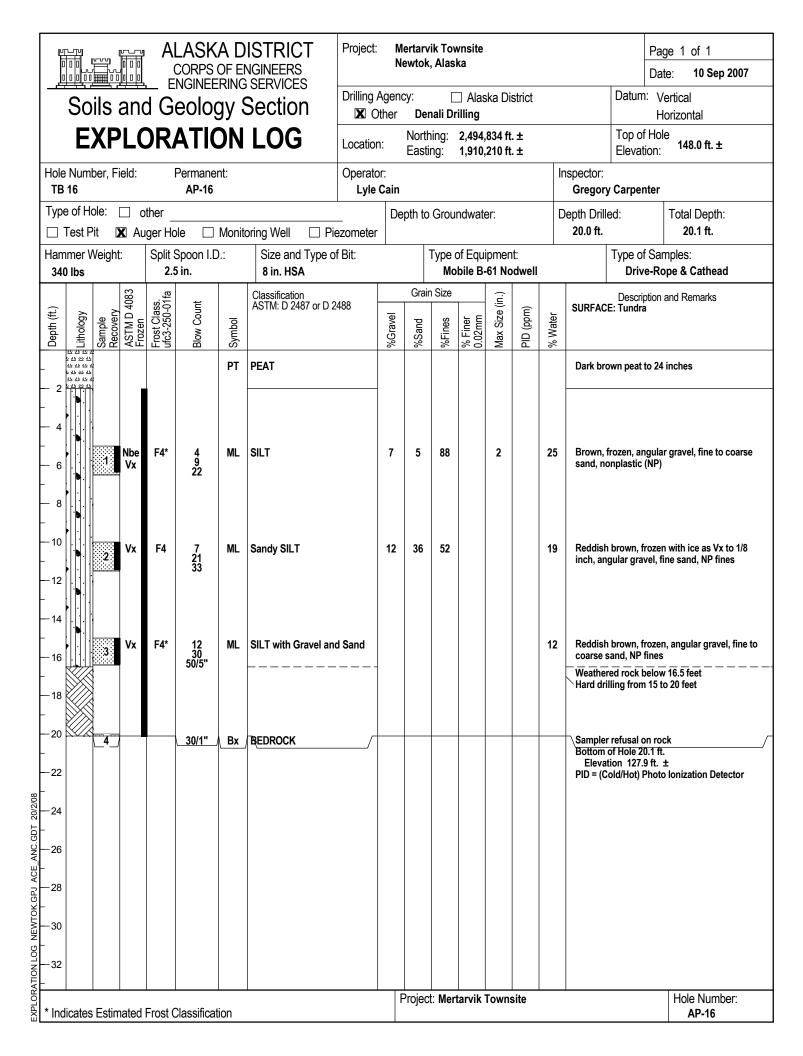


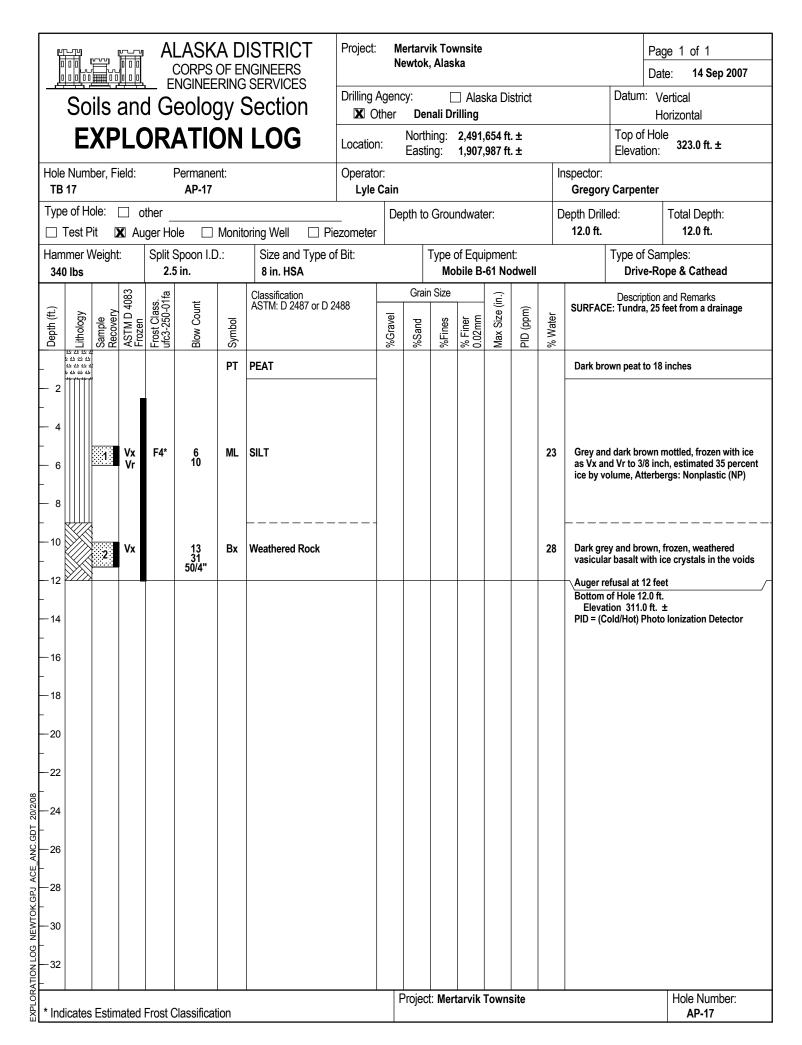


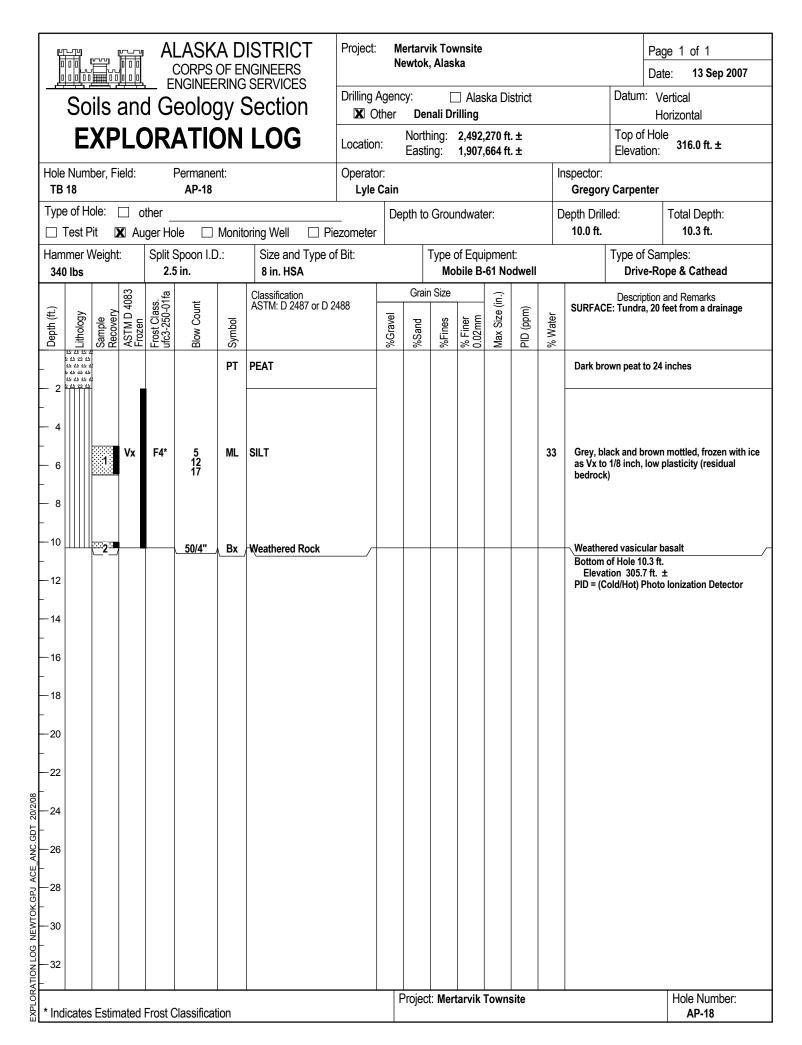


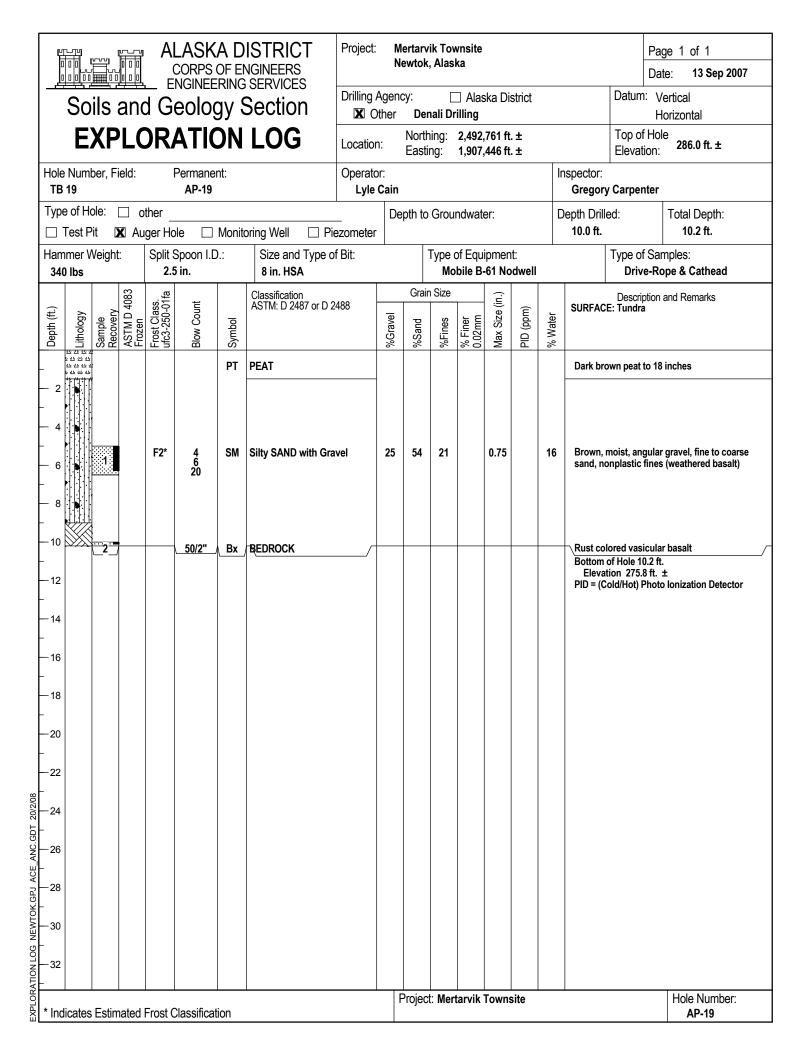


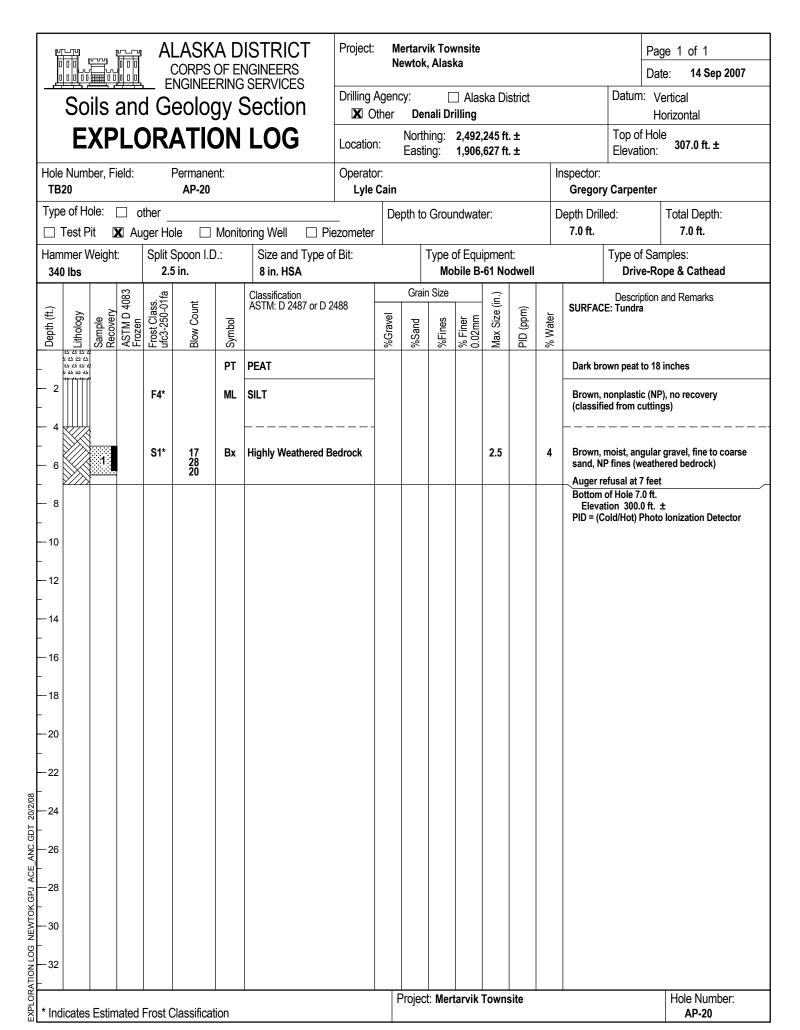


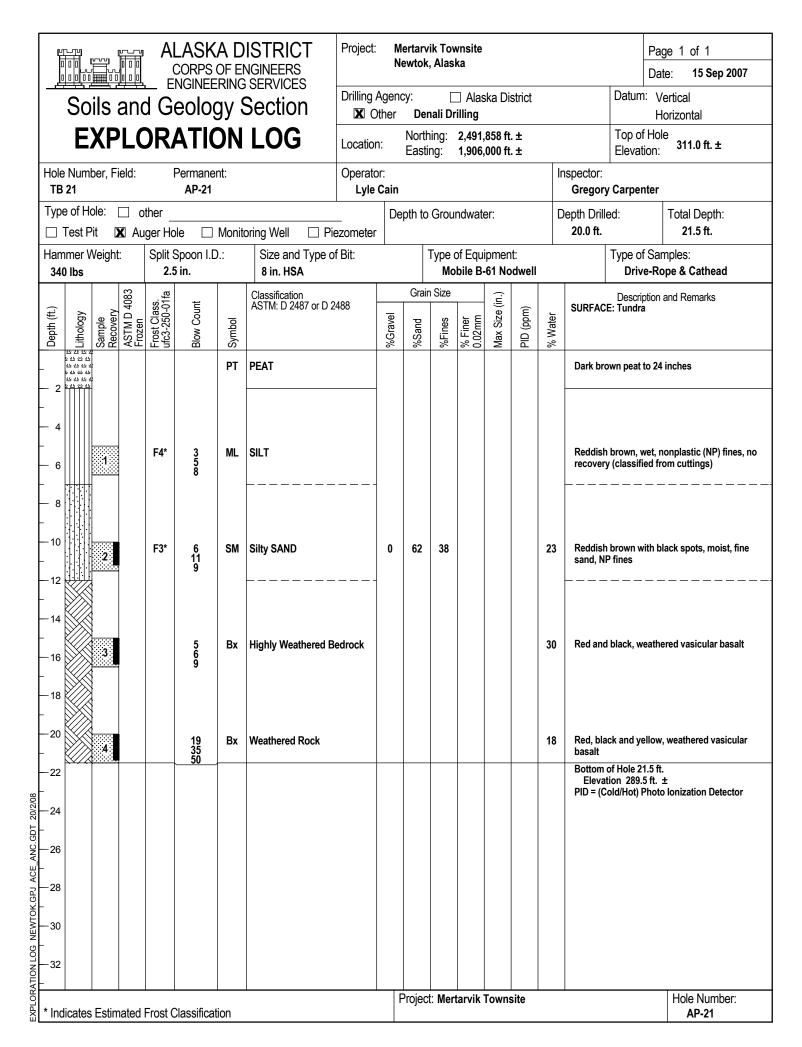


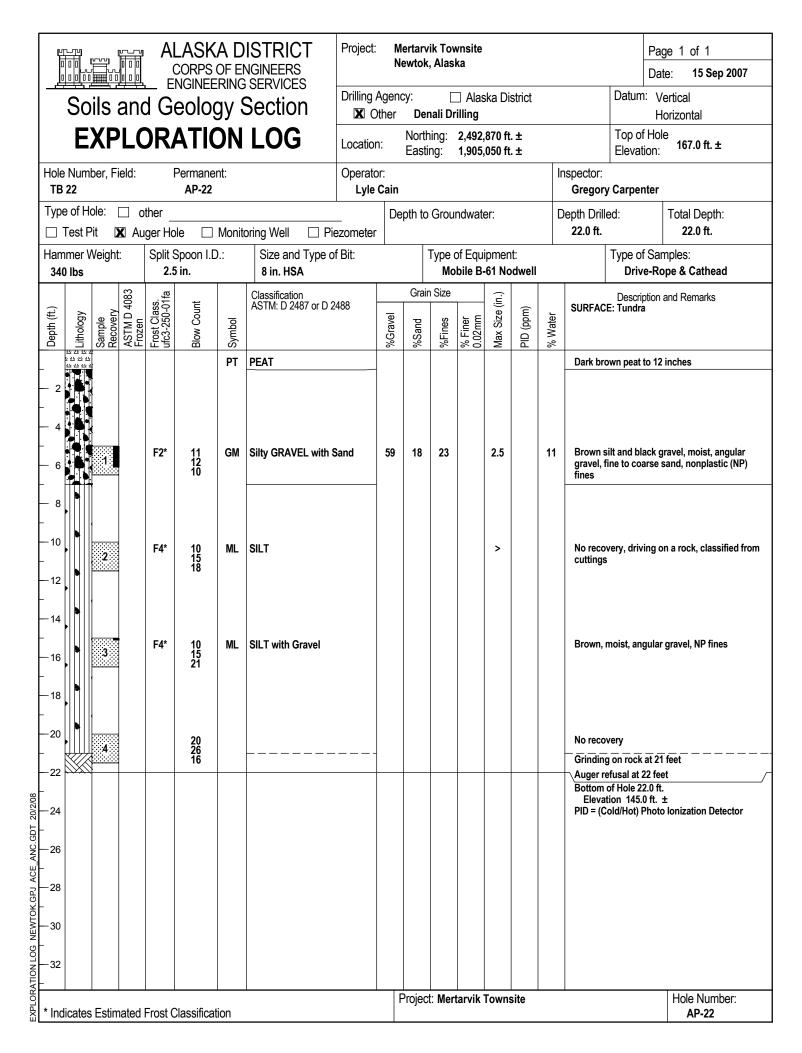


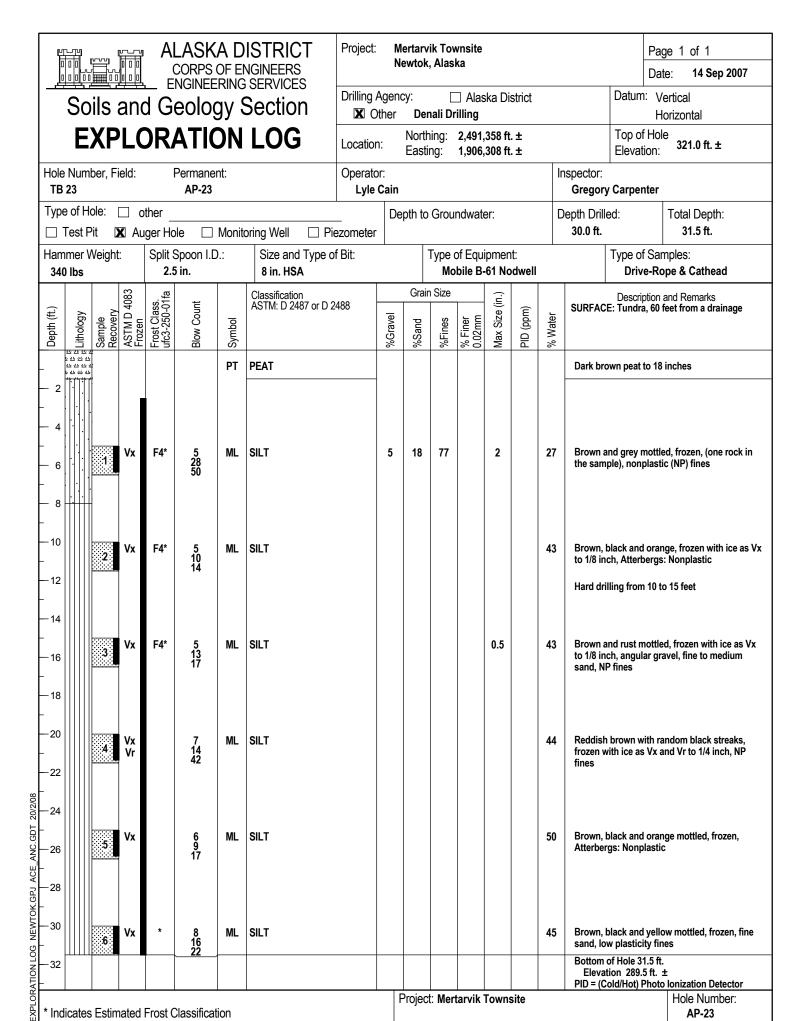






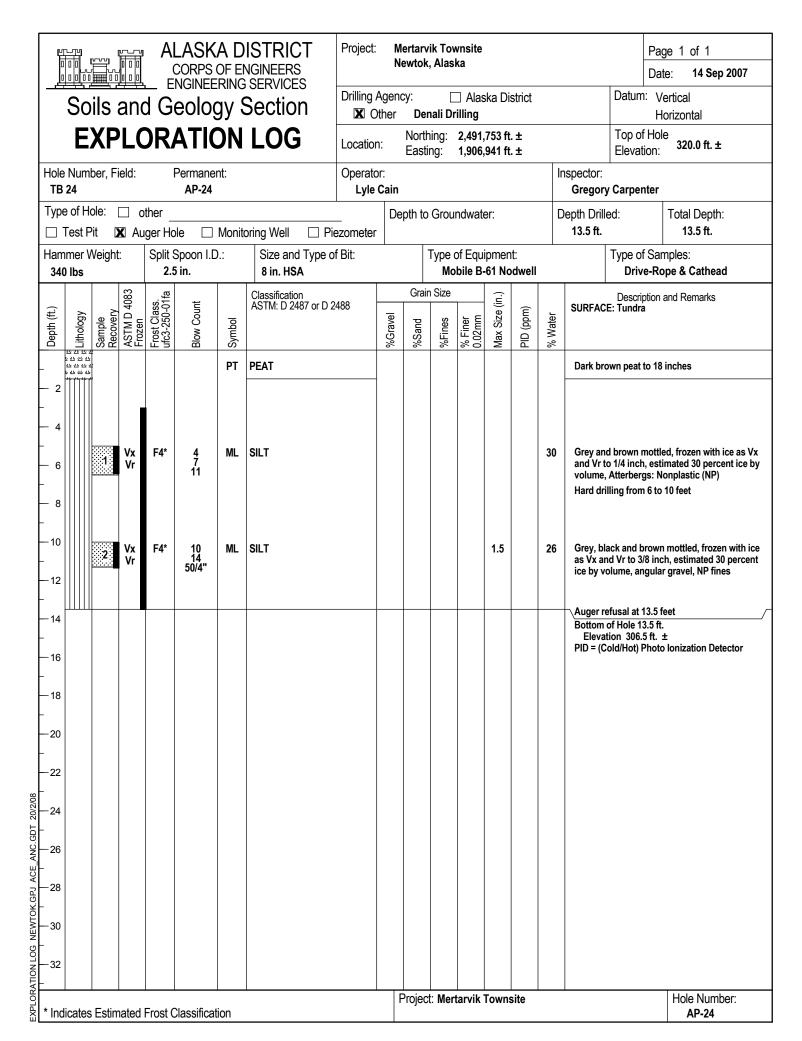






AP-23

Indicates Estimated Frost Classification



Appendix B Grain-size distribution curves

U.S. ARMY CORPS OF ENGINEERS SOILS AND GEOLOGY SECTION, ALASKA DISTRICT

Newtok Relocation

	Unified Soil Classification	
assing Frost	.02mm Class.	(%)
Passing F	-#200 0	(%)
Particle Size	Analysis	Gravel Sand Silt
imits	Ы	
rberg Li	Ы	
Atter	1	
Organic	Content	(%)
Moisture	Content	(%)
Interval	Bottom	
Depth	Тор	
	amble	Jumber
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			(GM) Silty gravel with sand					(ML) Sandy silt							(GM) Silty gravel					(GP-GM) Poorly graded gravel with silt		(ML) Sit	(ML) Silt with gravel			(ML) Sandy silt				(SM) Siky sand		(SM) Sifty sand	
9																																	
(\			3					2							4					2		6	9			2				2		2	
2			.8 36.3					.5 56.5							.4 35.4					7 9.5		5 88.9	7 73.6			.9 62.2				.9 44.2		.4 43.5	
ם ס			9 31.8) 42.5							2 10.4					8 9.7		3 7.5	7 7.7			36.9				48		1 45.4	
5			31.9					1.0							54.2					80.8		3.6	18.7			0.0				6.9		11.1	
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(0/)	5.7																																
(0/)	20.3	39.6		19.7	19.9	25.1	26.2		22.5	21.6	39.7	24.9	27.6	74.9		5.7	22.0	17.0	19.6	5.5	7.0		21.3	18.1	32.4	46.6		16.4	29.7	29.0	51.8	31.2	32.2
	3.5	6.5	11.5	3.5	6.5	3.5	6.5	11.5	3.5	6.5	3.5	6.5	3.5	6.5	11.5	3.5	6.5	3.5	6.5	3.5	3.5	5.8	3.5	6.5	3.5	6.5	6.5	11.0	6.5	11.0	6.5	11.5	16.8
	2.0	5.0	10.0	2.0	5.0	2.0	5.0	10.0	2.0	5.0	2.0	5.0	2.0	5.0	10.0	2.0	5.0	2.0	5.0	2.0	2.0	5.0	2.0	5.0	2.0	5.0	5.0	10.0	5.0	10.0	5.0	10.0	15.0
	SA-1	SA-2	SA-3	SA-1	SA-2	SA-1	SA-2	SA-3	SA-1	SA-2	SA-1	SA-2	SA-1	SA-2	SA-3	SA-1	SA-2	SA-1	SA-2	SA-1	SA-1	SA-2	SA-1	SA-2	SA-1	SA-2	SA-1	SA-2	SA-1	SA-2	SA-1	SA-2	SA-3
(1)	TB-1	TB-1	TB-1	TB-2	TB-2	TB-3	TB-3	TB-3	TB-4	TB-4	TB-5	TB-5	TB-6	TB-6	TB-6	TB-7	TB-7	TB-8	TB-8	TB-9	TB-10	TB-10	TB-11	TB-11	TB-12	TB-12	TB-13	TB-13	TB-14	TB-14	TB-15	TB-15	TB-15

U.S. ARMY CORPS OF ENGINEERS SOILS AND GEOLOGY SECTION, ALASKA DISTRICT

Newtok Relocation

	(ML) Silt	(ML) Sandy silt					(SM) Sity sand with gravel	(GP-GM) Well graded gravel with silt and sand	(SM) Silty sand			(GM) Silty gravel with sand	(ML) Silt with sand								
	88.0	51.7					21.5	7.3	38.0			22.5	76.9								
	4.6	36.2					53.5	25.9	62.0			18.0	18.0								
	7.4	12.1					25.0 5	66.8 2	0.0			59.5	5.1								
	7	1,					25	99	0			25	5								
				NP		NP								NP			AP		ΑN		
				_		_								_			_		_		
6	2	.7	4	0	2	2	8		6	2	6	.7	4	3	æ	7	6	5	0	9	
11.	25.	18.	12.	23.	28.	33.	15.	4.	22.	30.	17.	10.	27.	43.	42.	43.	49.	44.	30.	25.	
21.5	6.5	11.5	16.4	6.5	11.3	6.5	6.5	6.5	11.5	16.8	21.5	6.5	6.5	11.5	16.8	21.5	26.5	31.5	6.5	11.5	
20.0	5.0	10.0	15.0	5.0	10.0	5.0	5.0	5.0	10.0	15.0	20.0	5.0	5.0	10.0	15.0	20.0	25.0	30.0	5.0	10.0	
SA-4	SA-1	SA-2	SA-3	SA-1	SA-2	SA-1	SA-1	SA-1	SA-2	SA-3	SA-4	SA-1	SA-1	SA-2	SA-3	SA-4	SA-5	SA-6	SA-1	SA-2	
TB-15	TB-16	TB-16	TB-16	TB-17	TB-17	TB-18	TB-19	TB-20	TB-21	TB-21	TB-21	TB-22	TB-23	TB-23	TB-23	TB-23	TB-23	TB-23	TB-24	TB-24	

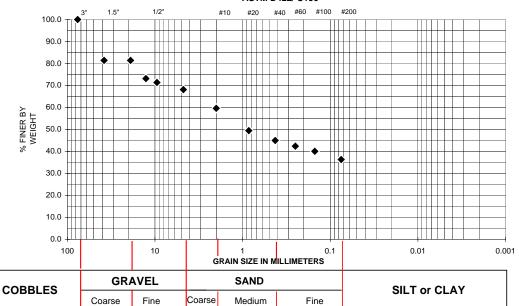
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-1
SAMPLE NO/ DEPTH	SA-3 (10.0'-11.5')
DESCRIPTION:	Silty gravel w/ sand
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 31.9	USC:	GM
% SAND: 31.8	FC:	
% SILT/CLAY: 36.3 .0)2 mm:	
·		
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	12.7	

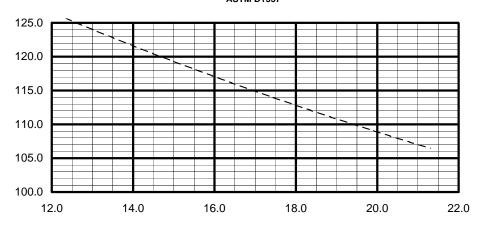
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"	100	
38.1	1.5"	81	
19.05	3/4"	81	
12.7	1/2"	73	
9.5	3/8"	71	
4.75	# 4	68	
2	#10	60	
0.85	#20	49	
0.425	#40	45	
0.25	# 60	42	
0.15	#100	40	
0.075	#200	36.3	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

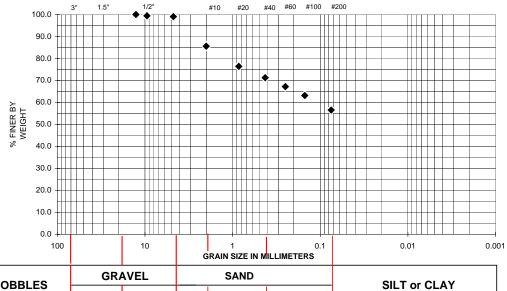
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-3
SAMPLE NO/ DEPTH	SA-3 (10.0'-11.5')
DESCRIPTION:	Sandy silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 1.0 % SAND: 42.5	USC:_ FC:_	ML
	02 mm:	
ASTM D1557(uncorrected) ASTM D4718 (corrected)		ocf ocf
OPTIMUM M.C.% (corrected)		- - ·
NATURAL M.C. %	23.8	

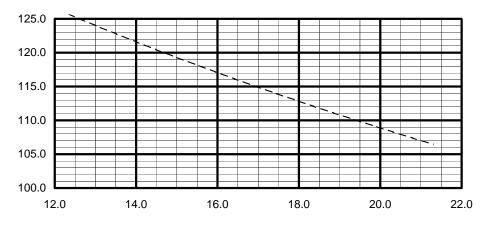
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEV	E ANALY	SIS RES	ULT
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	99	
4.75	# 4	99	
2	#10	86	
0.85	#20	76	
0.425	#40	71	
0.25	# 60	67	
0.15	#100	63	
0.075	#200	56.5	

COBBLES Coarse Coarse Medium

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

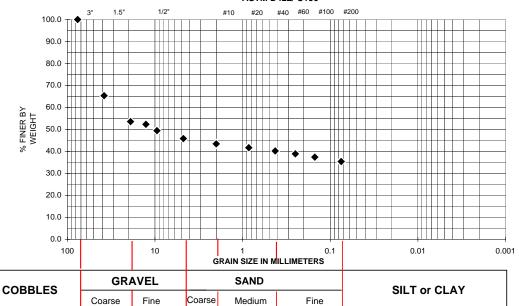
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-6
SAMPLE NO/ DEPTH	SA-3 (10.0'-11.5')
DESCRIPTION:	Silty gravel
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL:	54.2	_	USC:	GM
% SAND:	10.4	_	FC:	
% SILT/CLAY:	35.4	.02	mm:	
			_	
ASTM D1557(uncorr	ected)			pcf
ASTM D4718 (corr	ected)			pcf
OPTIMUM M.C.% (c	orrected)		•	·
NATURAL M.C. %			10.3	

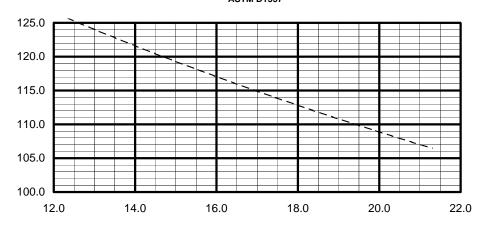
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"	100	
38.1	1.5"	65	
19.05	3/4"	54	
12.7	1/2"	52	
9.5	3/8"	49	
4.75	# 4	46	
2	#10	43	
0.85	#20	42	
0.425	#40	40	
0.25	# 60	39	
0.15	#100	37	
0.075	#200	35.4	
	-	-	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

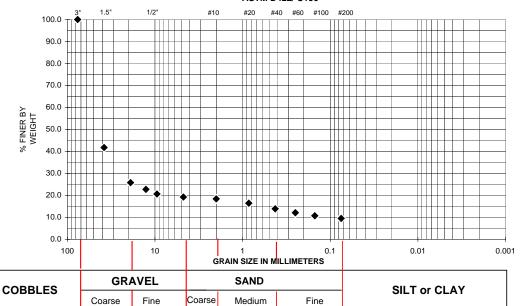
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-9
SAMPLE NO/ DEPTH	SA-1 (2.0'-3.5')
DESCRIPTION:	Poorly grd. gravel w/ silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 80.8	USC: GP-GM
% SAND: 9.7	FC:
% SILT/CLAY: 9.5	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	5.5

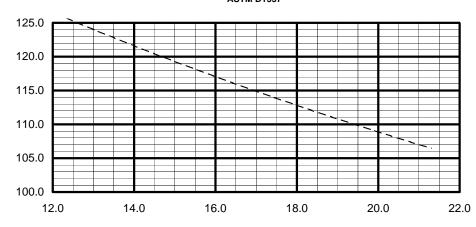
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

CILTE ANALTOIC RECOLT			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"	100	
38.1	1.5"	42	
19.05	3/4"	26	
12.7	1/2"	23	
9.5	3/8"	21	
4.75	# 4	19	
2	#10	18	
0.85	#20	16	
0.425	#40	14	
0.25	# 60	12	
0.15	#100	11	
0.075	#200	9.5	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-10
SAMPLE NO/ DEPTH	SA-2 (5.0'-8.3')
DESCRIPTION:	Silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

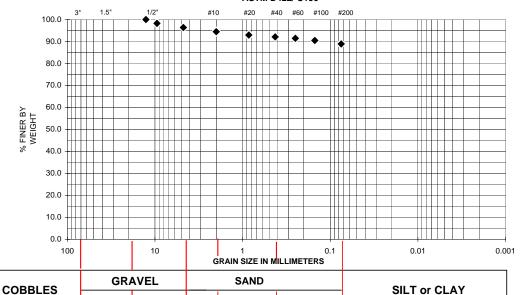
Coarse

Fine

Coarse

% GRAVEL: 3.6 % SAND: 7.5	USC: FC:	ML
	02 mm:	,
ASTM D1557(uncorrected) ASTM D4718 (corrected)	p	
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	22.5	

PARTICLE SIZE ANALYSIS ASTM D422/ C136

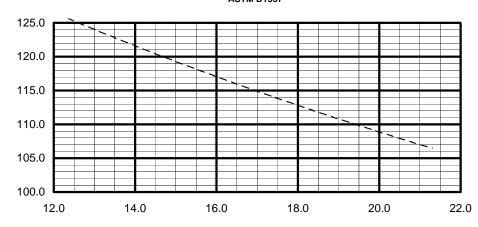


SIEVE ANALYSIS RESULT

OILTE ANALTOIC RECOLT			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	98	
4.75	# 4	96	
2	#10	94	
0.85	#20	93	
0.425	#40	92	
0.25	# 60	91	
0.15	#100	90	
0.075	#200	88.9	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557

Medium



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

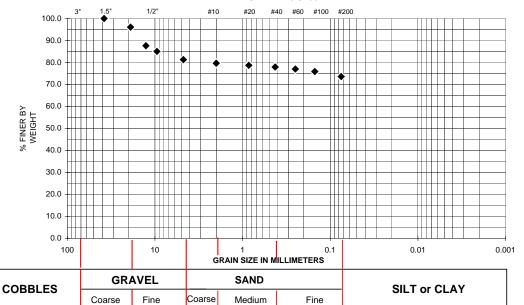
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-11
SAMPLE NO/ DEPTH	SA-1 (2.0'-3.5')
DESCRIPTION:	Silt with gravel
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 18.7 % SAND: 7.7 % SILT/CLAY: 73.6	USC: ML FC: 02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	21.3

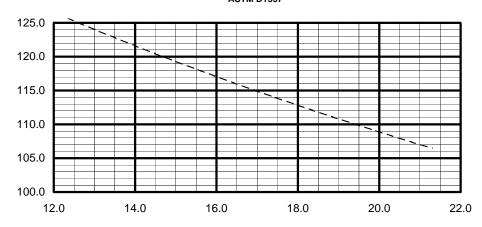
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	96	
12.7	1/2"	88	
9.5	3/8"	85	
4.75	# 4	81	
2	#10	80	
0.85	#20	79	
0.425	#40	78	
0.25	# 60	77	
0.15	#100	76	
0.075	#200	73.6	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

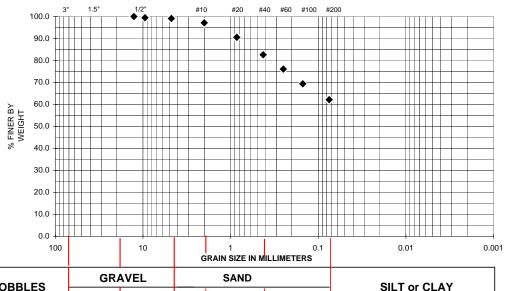
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-12
SAMPLE NO/ DEPTH	SA-2 (5.0'-6.5')
DESCRIPTION:	Sandy silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 0.9	USC: FC:	ML
% SILT/CLAY: 62.2 .0)2 mm:	
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	46.6	

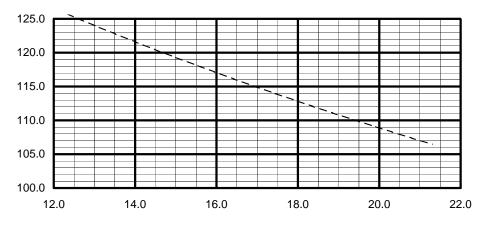
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT				
SIEVE	SIEVE	TOTAL %		
SIZE (mm)	SIZE (in.)	PASSING	SPEC	
152.4	6"			
76.2	3"			
38.1	1.5"			
19.05	3/4"			
12.7	1/2"	100		
9.5	3/8"	99		
4.75	# 4	99		
2	#10	97		
0.85	#20	91		
0.425	#40	83		
0.25	# 60	76		
0.15	#100	69		
0.075	#200	62.2		

COBBLES SILT or CLAY Coarse Fine Coarse Medium

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250	•	
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

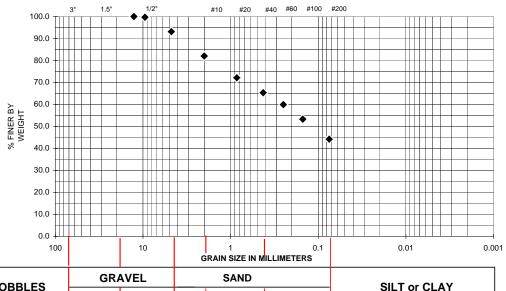
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-14
SAMPLE NO/ DEPTH	SA-2 (10.0'-11.5')
DESCRIPTION:	Silty sand.
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 6.9	USC: SM
% SAND: 48.9	FC:
% SILT/CLAY: 44.2	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	29.0

PARTICLE SIZE ANALYSIS ASTM D422/ C136

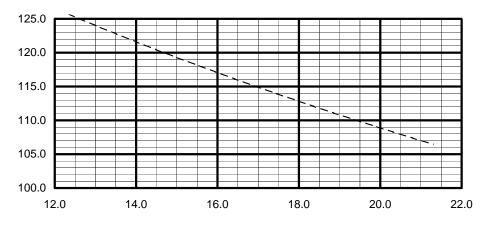


SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	100	
4.75	# 4	93	
2	#10	82	
0.85	#20	72	
0.425	#40	65	
0.25	# 60	60	
0.15	#100	53	

SIEVE ANALYSIS RESULT

COBBLES SILT or CLAY Coarse Fine Coarse Medium

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

#200

44.2

0.075

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

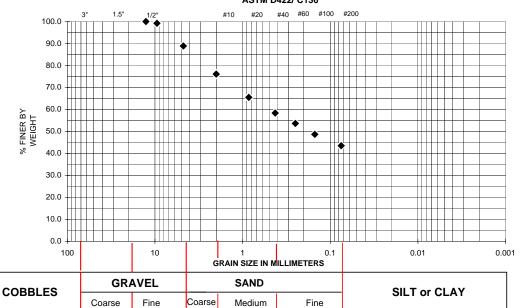
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-15
SAMPLE NO/ DEPTH	SA-2 (10.0'-11.5')
DESCRIPTION:	Silty sand.
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 11.1	USC: SM
% SAND: 45.4	FC:
% SILT/CLAY: 43.5	02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	31.2

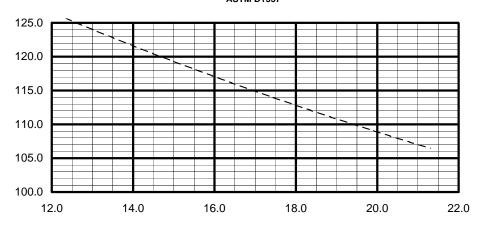
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	99	
4.75	# 4	89	
2	#10	76	
0.85	#20	66	
0.425	#40	58	
0.25	# 60	54	
0.15	#100	49	
0.075	#200	43.5	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

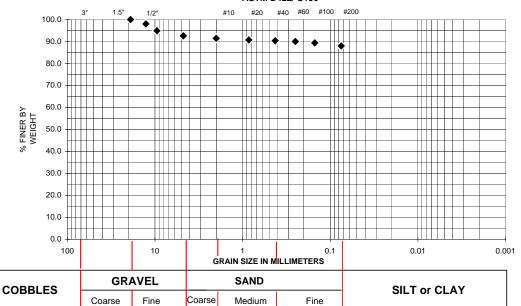
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-16
SAMPLE NO/ DEPTH	SA-1 (5.0'-6.5')
DESCRIPTION:	Silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

0/ ODANEL - 7.4	1100
% GRAVEL: 7.4	USC: ML
% SAND: 4.6	FC:
% SILT/CLAY: 88.0	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	25.2

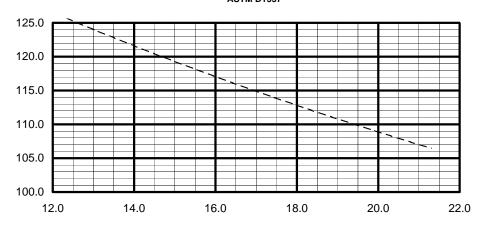
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	98	
9.5	3/8"	95	
4.75	# 4	93	
2	#10	91	
0.85	#20	91	
0.425	#40	90	
0.25	# 60	90	
0.15	#100	89	
0.075	#200	88.0	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

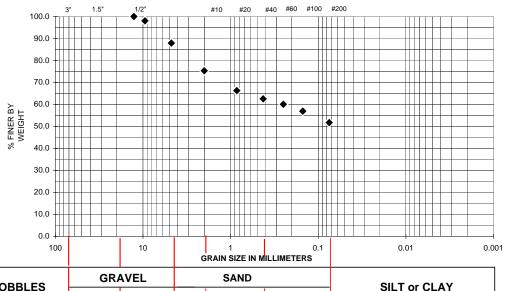
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-16
SAMPLE NO/ DEPTH	SA-2 (10.0'-11.5')
DESCRIPTION:	Sandy silt
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 12.1	USC:	ML
% SAND: 36.2	FC:	
% SILT/CLAY: 51.7 .0	2 mm:	
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		•
NATURAL M.C. %		

PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	98	
4.75	# 4	88	
2	#10	75	
0.85	#20	66	
0.425	#40	63	
0.25	# 60	60	

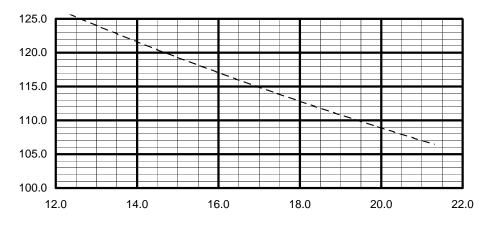
57

51.7

SIEVE ANALYSIS RESULT

			9	RAIN SIZE IN W	ILLIMETERS	
COBBLES	GR/	AVEL	EL SAND		SILT or CLAY	
COBBLES	Coarse	Fine	Coarse	Medium	Fine	SILT OF CLAT

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

#100

#200

0.15

0.075

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

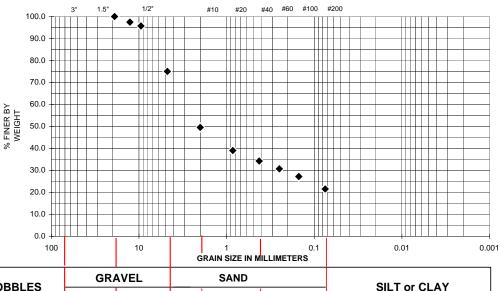
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-19
SAMPLE NO/ DEPTH	SA-1 (5.0'-6.5')
DESCRIPTION:	Silty sand w/ gravel
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 25.0	USC:	SM
% SAND: 53.5	FC:	
% SILT/CLAY: 21.5 .()2 mm:	
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	15.8	

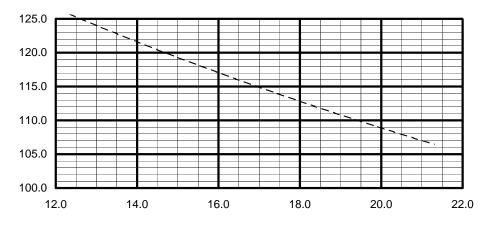
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT				
SIEVE	SIEVE	TOTAL %		
SIZE (mm)	SIZE (in.)	PASSING	SPEC	
152.4	6"			
76.2	3"			
38.1	1.5"			
19.05	3/4"	100		
12.7	1/2"	97		
9.5	3/8"	96		
4.75	# 4	75		
2	#10	50		
0.85	#20	39		
0.425	#40	34		
0.25	# 60	31		
0.15	#100	27		
0.075	#200	21.5		

COBBLES SILT or CLAY Coarse Fine Coarse Medium

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

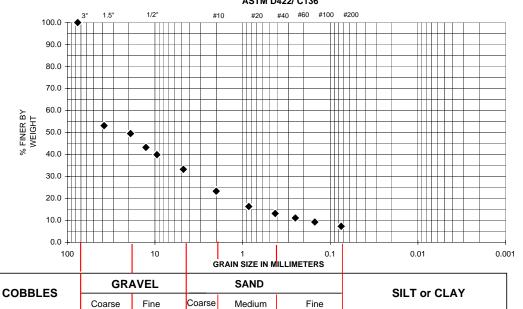
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-20
SAMPLE NO/ DEPTH	SA-1 (5.0'-6.5')
DESCRIPTION:	Well grd. gravel w/ silt & sand
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 66.8 % SAND: 25.9 % SILT/CLAY: 7.3	USC: GW-GM FC: .02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	4.1

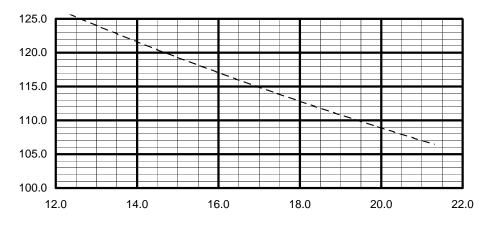
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"	100	
38.1	1.5"	53	
19.05	3/4"	49	
12.7	1/2"	43	
9.5	3/8"	40	
4.75	# 4	33	
2	#10	23	
0.85	#20	16	
0.425	#40	13	
0.25	# 60	11	
0.15	#100	9	
0.075	#200	7.3	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

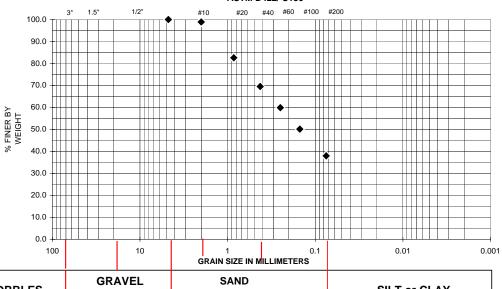
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-21
SAMPLE NO/ DEPTH	SA-2 (10.0'-11.5')
DESCRIPTION:	Silty sand.
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 0.0	USC:	SM
% SAND: 62.0	FC:	
% SILT/CLAY: 38.0 .0	2 mm:	
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		·
NATURAL M.C. %	22.9	

PARTICLE SIZE ANALYSIS ASTM D422/ C136



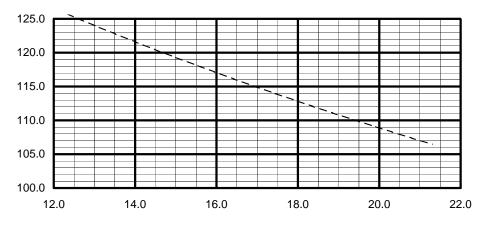
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"		
9.5	3/8"		
4.75	# 4	100	
2	#10	99	
0.85	#20	83	
0.425	#40	70	
0.25	# 60	60	
0.15	#100	50	

38.0

SIEVE ANALYSIS RESULT

COBBLES SILT or CLAY Coarse Fine Coarse Medium

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

#200

0.075

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

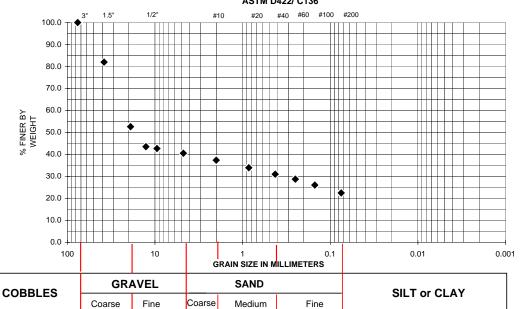
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-22
SAMPLE NO/ DEPTH	SA-1 (5.0'-6.5')
DESCRIPTION:	Silty gravel w/ sand
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 59.5	USC: GM	
% SAND: 18.0	FC:	
% SILT/CLAY: 22.5	.02 mm:	
ASTM D1557(uncorrected)	pcf	
ASTM D4718 (corrected)	pcf	
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	10.7	

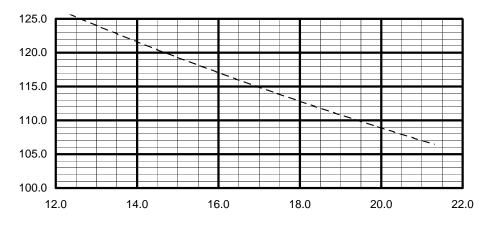
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"	100	
38.1	1.5"	82	
19.05	3/4"	53	
12.7	1/2"	44	
9.5	3/8"	43	
4.75	# 4	41	
2	#10	37	
0.85	#20	34	
0.425	#40	31	
0.25	# 60	29	
0.15	#100	26	
0.075	#200	22.5	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

Laboratory Testing / Construction Monitoring

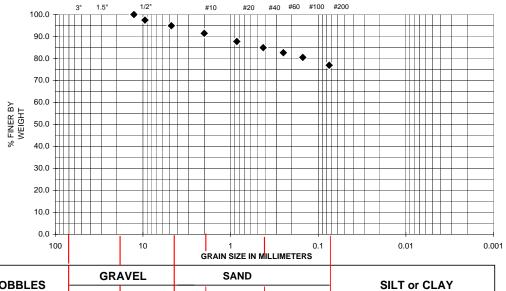
Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	1811-07
SAMPLE LOCATION:	TB-23
SAMPLE NO/ DEPTH	SA-1 (5.0'-6.5')
DESCRIPTION:	Silt with sand
DATE TESTED:	11/7/2007
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 5.1	USC:	ML
% SAND: 18.0	FC:	
% SILT/CLAY: 76.9	.02 mm:	
	1	
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %		

PARTICLE SIZE ANALYSIS





SIEVE ANALYSIS RESULT				
SIEVE	SIEVE	TOTAL %		
SIZE (mm)	SIZE (in.)	PASSING	SPEC	
152.4	6"			
76.2	3"			
38.1	1.5"			
19.05	3/4"			
12.7	1/2"	100		
9.5	3/8"	97		
4.75	# 4	95		
2	#10	91		
0.85	#20	88		
0.425	#40	85		
0.25	# 60	83		
0.15	#100	80		
0.075	#200	76.9		

COBBLES Coarse Fine Coarse Medium

HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557

125.0		`\																Ì
120.0			<u> </u>	, /	_													
115.0						<i>,</i>	í	`										
110.0									,	/	```	_	/					
105.0														Í	,′	,		
100.0 1	.0		14	.0		16	5.0		18	3.0	ı		20	0.0			22	.0

ADDENDUM

GEOTECHNICAL REPORT MERTARVIK TOWNSITE NEWTOK, ALASKA

January 2009

ADDENDUM

GEOTECHNICAL REPORT MERTARVIK TOWNSITE NEWTOK, ALASKA

JANUARY 2009

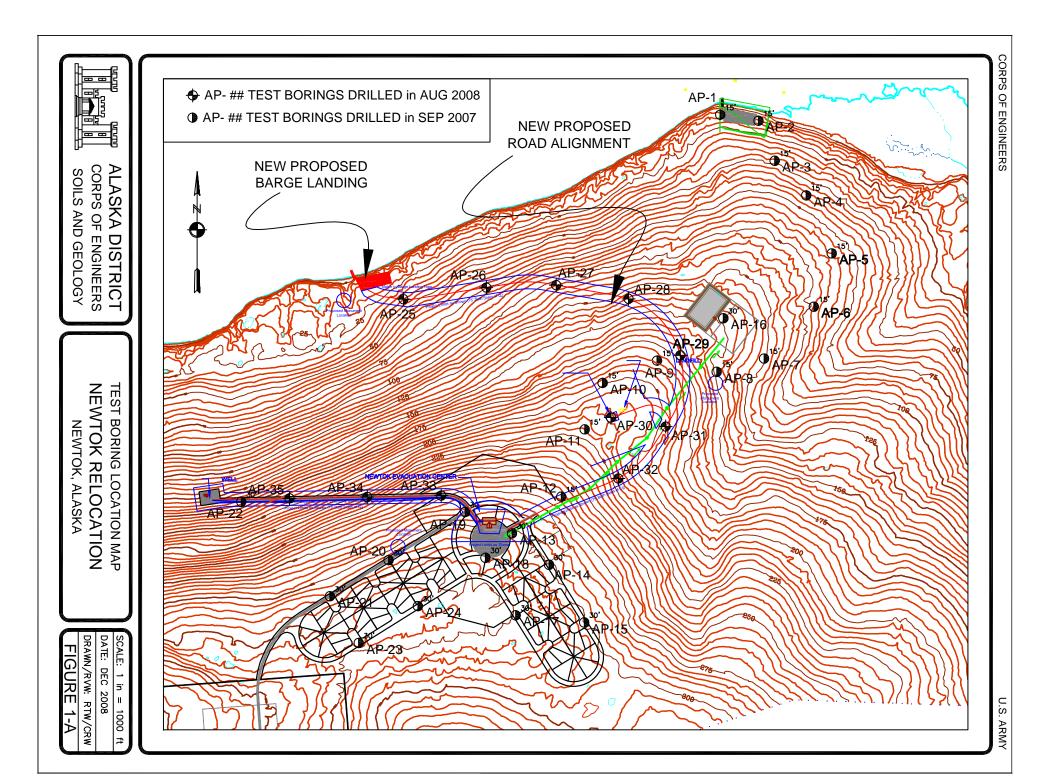
Since the exploration was performed and the report prepared during the fall of 2007 and the spring of 2008 the location of the barge landing has been relocated. This resulted in the realignment of the road leading from the barge landing to the proposed town site. A total of 11 new borings were drilled, sampled and logged along the new proposed roadway alignment from 20 to 21 August 2008. The locations of the borings were located and staked by standard survey methods by R&M Engineers under contract to USACE-AD prior to the drilling. A map showing the locations of the borings is attached as Figure A-1. Copies of the exploration logs are attached in Appendix Addendum – Exploration Logs.

Laboratory testing was performed on selected samples from the exploration. The results of those tests are presented on the exploration logs and in the Appendix Addendum – Grain-Size Distribution Curves.

The results of the additional exploration were generally consistent with the previous exploration. The subsurface conditions generally consist of about two feet of surface organics over silt with some sand and gravel (ML). The soils are a product of weathering of the underlying basalt rock and generally have a frost classification of F4. The silts generally contain more sand and gravel with depth and become more competent as the rock surface is approached. In general the rock surface is encountered between ten and 15 feet below grade.

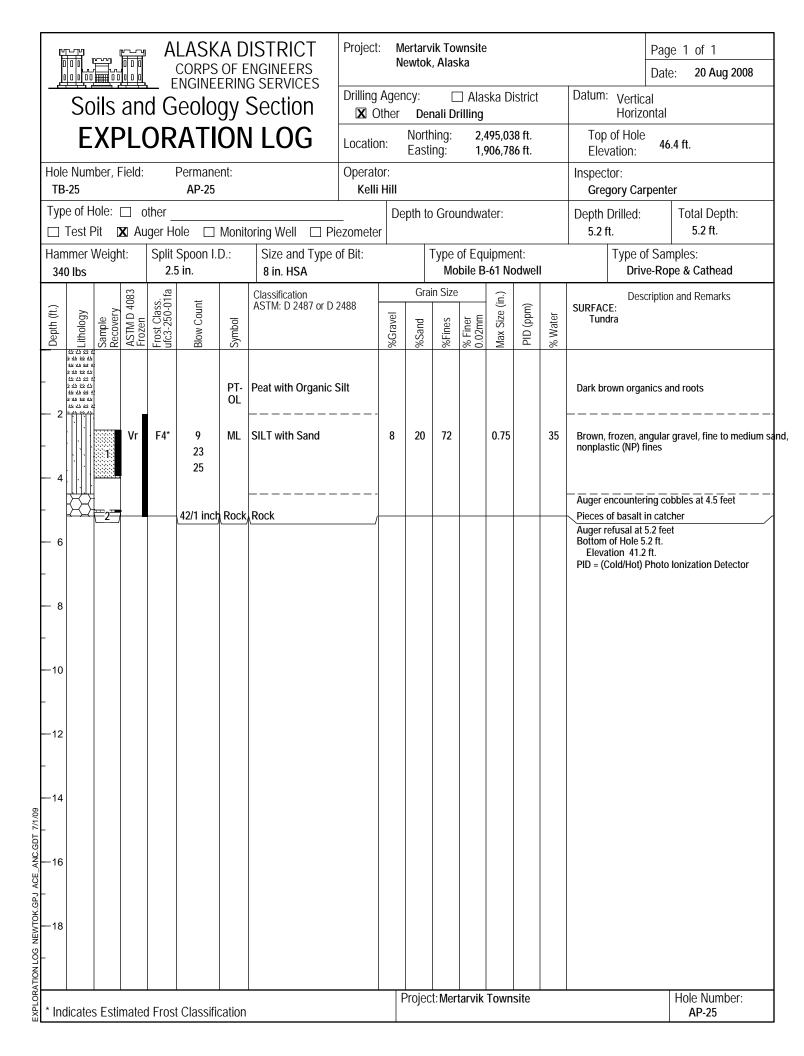
The entire area is generally wet with water in the organic mat. Intermittent permafrost is present in the area and was encountered in several of the borings.

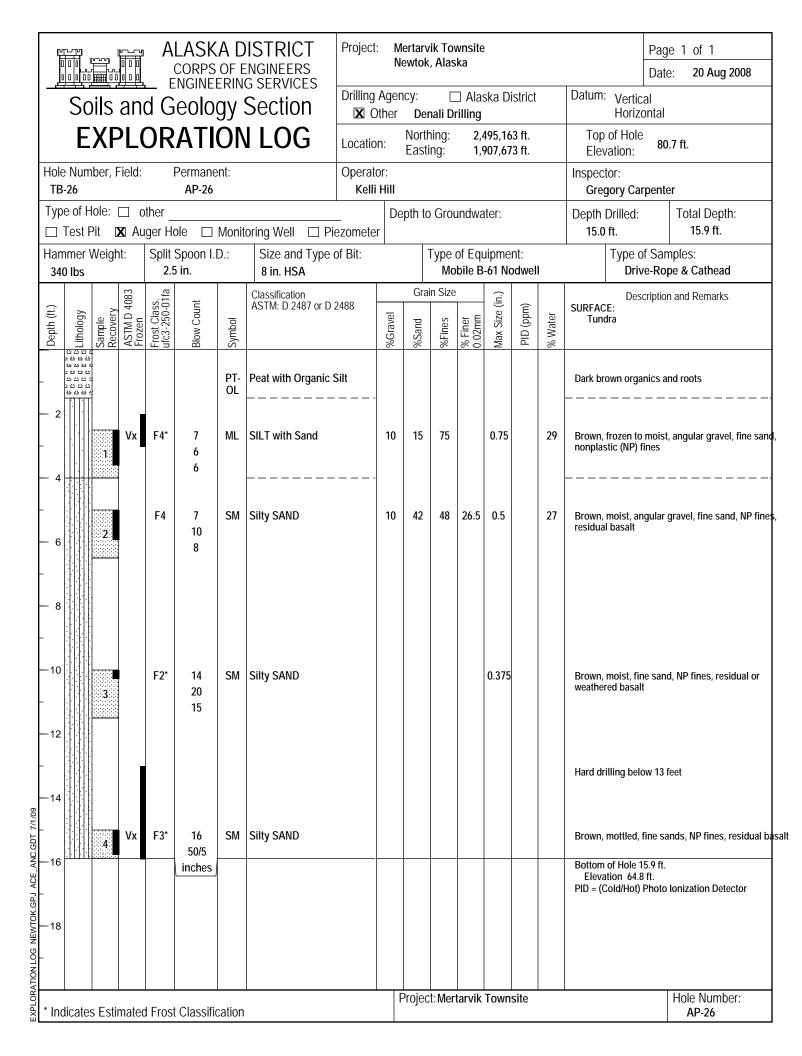
None of the findings of this exploration effort changes the findings or recommendations of the previous exploration.



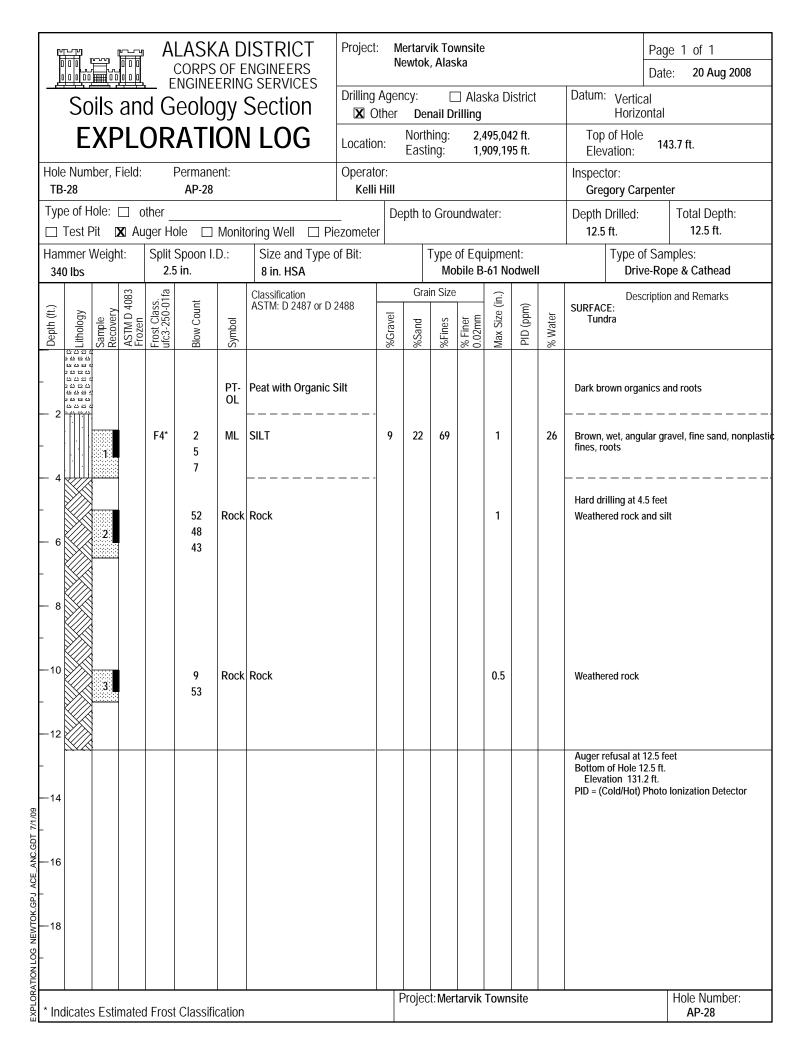
APPENDIX ADDENDUM

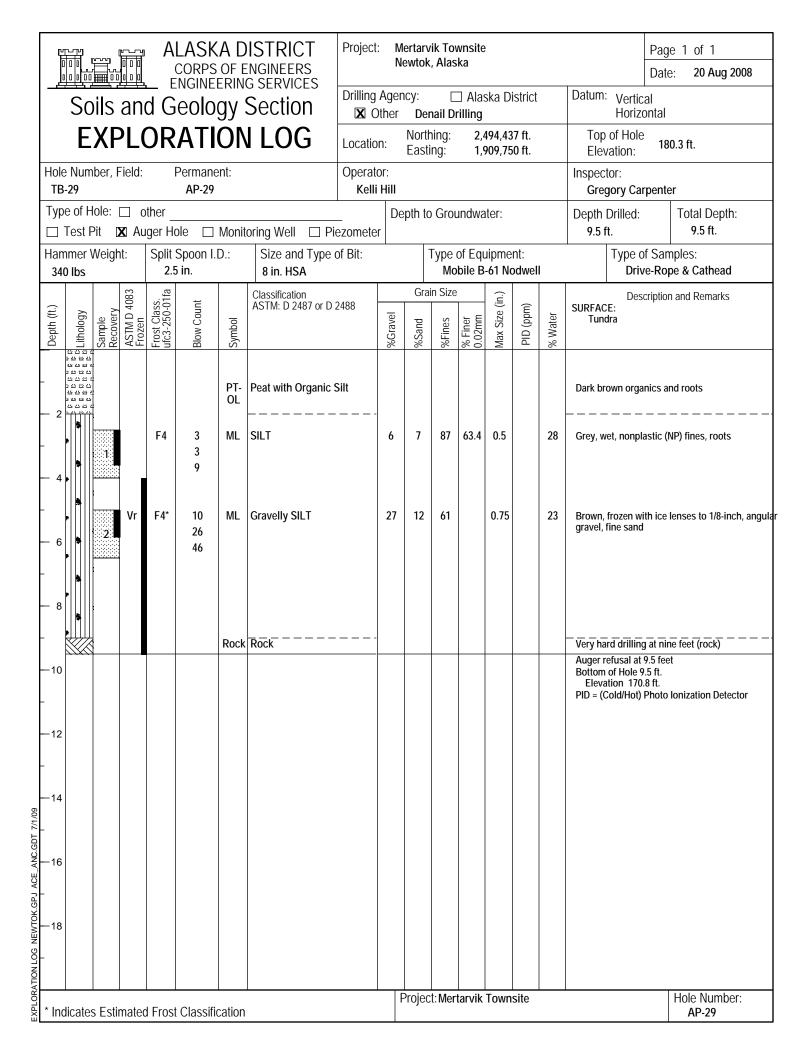
Exploration Logs

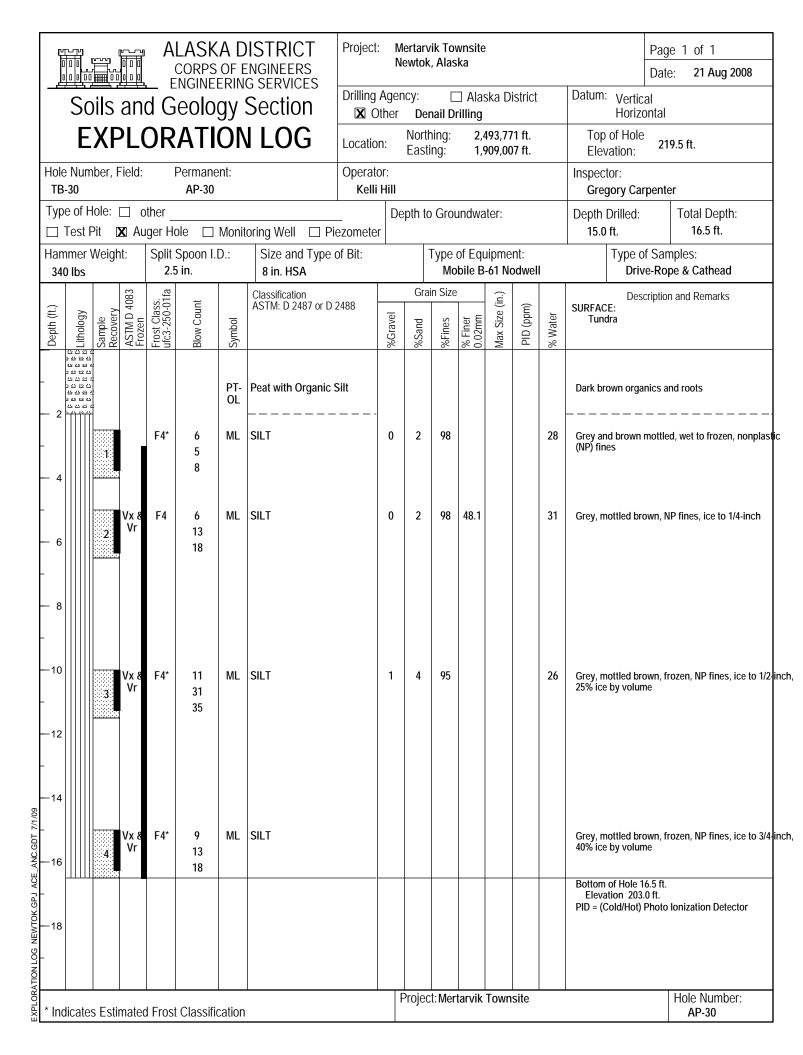




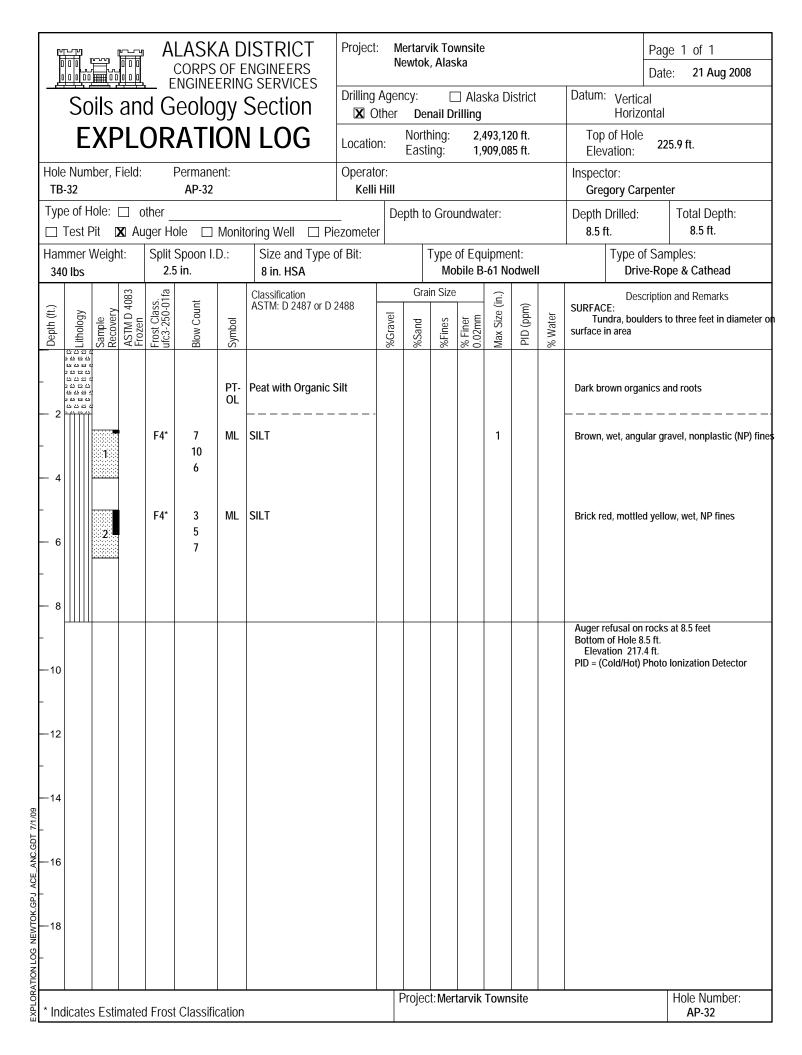
<u>.</u>			0	CORPS	OF E	ISTRICT ENGINEERS G SERVICES	Project		ertarv ewtok		wnsite ska)					Page	e 1 of 1 e: 20 Aug 2008
	Sc	oils ar				Section	Drilling	Agen Other	-		☐ Ala	ska D	istrict		Datum	: Vertic Horiza	al ontal	
	E	XPL	OR	ATI	OI\	I LOG	Locatio	n:	North Easti	11	1.9 ft.							
Hole		nber, Field	d:	Permane AP-27	ent:		Operat Kelli								Inspec Gre	tor: gory Car	pente	er
1 .		Hole: Pit	_	ole 🗆	Monit	oring Well □ Pi	– ezomete		pth to	Grou	undwa	ater:			Depth 15.0	Drilled: ft.		Total Depth: 15.3 ft.
Han		Weight:	Split	Spoon I. 5 in.		Size and Type 6			-			uipme 5-61 No		I		Type o Driv		nples: De & Cathead
		1083	ss. 01fa	ıt		Classification ASTM: D 2487 or D	2/188		Gra	in Size	:	(in.)			CHDEAC	Desc	cription	n and Remarks
Depth (ft.)	្តេ ដែlthology	Sample Recovery ASTM D 4083	Frost Class. ufc3-250-01fa	Blow Count	Symbol	ASTIVI. D 2407 01 D	2400	%Gravel	%Sand	%Fines	% Finer 0.02mm	Max Size (in.)	PID (ppm)	% Water	SURFAC Tun			
- 2	는 다 한 한 는 항 한 한 는 항 한 한 는 항 한 한 는 한 한 한 는 한 한 는 한 한 는 한 한 는 한 한 는 한 는	4			PT- OL	Peat with Organic S	Silt								Dark br	own orga	nics ar	nd roots
- - 4		3333	F4*	3 3 8	ML	Sandy SILT with G	ravel 	16	20	64		0.75		43	Brown, (NP) fin	wet, angues, roots	ılar gra	avel, fine sand, nonplastic
- - 6 - - 8		2	F3*	4 5 7	SM	Silty SAND with Gr	ravel	20	39	41		0.5		43	sand, N	yellow an IP fines, w Limit= No	reather	nge, wet, angular gravel, f red or residual basalt ic
- -10 - -12		3	F3	16 30 25	SM	Silty SAND with Gr	ravel	15	49	36	15.7	0.375		24	fine sar	h brown, i nd, NP to l Limit= No	low pla	d yellow, dry, angular gra asticity fines, residual bas ic
- -14							. — — -								Hard dr	illing belo	₩ 13 f	eet
EXPLORATION LOG NEWTOK. GPJ ACE_ANC. GDT 7/1/09 *		3343.		50/3 inches	Rock	Rock									Bottom Eleva	of Hole 1 ation 96.6	5.3 ft. ft.	red in sampler. Ionization Detector
TION LOG N																		
* Inc	licate	s Estimat	ed Fros	t Classifi	cation	1		F	rojec	t:Mer	tarvik	Town	site					Hole Number: AP-27







	0 0					C	ORPS	OF E	ISTRICT ENGINEERS G SERVICES	Project		ertarv ewtok		wnsite ska	!					Pag Date	e 1 of 1 e: 21 Aug 2008
-	,	Sc	oils	ar	<u>-</u> nd				Section	Drilling	Agen Other	-		☐ Alas	ska D	istrict		Datum	: Vertic Horizo	al ontal	
		E	XI	PΓ	0	R/	ATI	Ô۱	I LOG	Locatio	n:	North Easti			493,67 909,59				of Hole	20	4.3 ft.
	le l		nber,	Field	l:		ermane AP-31	nt:		Operat Kelli								Inspec	tor: gory Car	pente	er
ľ	•				othe luger		e 🗆	Monit	oring Well □ Pi	ezomete		pth to	Grou	undwa	iter:			Depth 15.0	Total Depth: 16.5 ft.		
На		ner	Weig		Sp		oon I.I		Size and Type 8 in. HSA			-		of Equ			l		Type of Samples: Drive-Rope & Cathead		
							Classification ASTM: D 2487 or D	2400		Gra	in Size)				OUDEAG	Description and R				
Depth (ft.)	-	Lithology	Sample	ASTM D 4083	Frost Class.	ıfc3-250-(Blow Count	Symbol	ASTIVI. D 2407 01 D	2400	%Gravel	%Sand	%Fines	% Finer 0.02mm	Max Size (in.)	PID (ppm)	% Water	SURFAC Tun			
-	22 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	는 10 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	호 8 8 8				1	PT- OL	Peat with Organic	Silt					-		0	Dark br	own orga	nics ar	nd roots
-	Δ 4		1		F4	1 *	9 9 11	ML	Gravelly SILT with	Sand	27	23	50		1		30	Brown, (NP) fin		ıgular (gravel, fine sand, nonplast
_	6 \(\(\triangle \)		2	<u> </u>	F	2	7 12 14	GM	Silty GRAVEL with	Sand	41	31	28	18.4	1		26	Brown,	moist, an	ngular (gravel, fine sand, NP fines
- 1 1			3	Vr	F4	1 *	18 33 42	ML	SILT with Sand		8	18	74		0.5		35	NP fine			n, angular gravel, fine san
ACE_ANC.GDT 7/1/09			4	Vr	F4	1 *	11 15 23	ML	SILT										ed, frozen		lasticity, 30% ice by volum
EXPLORATION LOG NEWTOK.GPJ ACE_ANC.GDT 7/1/09		nato	e Fe	imate	ad Fr	nst (Classifid	ration			F	Projec	t:Mer	tarvik	Town	site		Eleva	ation 187.	.8 ft.	Hole Number: AP-31



<u> </u>					CORPS	OF E	ISTRICT ENGINEERS	Project			vik To k, Alas	wnsite ska	·				Pag	ge 1 of 1 e: 21 Aug 2008		
<u></u>	So	ils	and				g services Section	Drilling	Ager Other	-		☐ Ala:	ska D	istrict		Datum: Verti		- J		
							I LOG	Locatio			hing:	2,	492,93 907,19	Top of Hol Elevation:	Top of Hole Elevation: 279.4 ft.					
Hole N		ber, F	ield:		Permane AP-33	ent:		Operat Kell								Inspector: Gregory Ca	arpent	er		
Type □ Te				_	ole 🗆	Monit	oring Well 🔲 Pi	 iezomete		epth to	o Gro	undwa	iter:			Depth Drilled: 10.0 ft.		Total Depth: 10.3 ft.		
	Hammer Weight: Split Spoon I.D.: 2.5 in.					Size and Type 8 in. HSA	of Bit:				of Equ			ı			mples: pe & Cathead			
Depth (ft.)	ogy	le ⁄ery	ASTM D 4083 Frozen	Frost Class. ufc3-250-01fa	Blow Count	lo	Classification ASTM: D 2487 or D	2488	vel		ain Size		Max Size (in.)	PID (ppm)	ter	De SURFACE: Tundra	scriptio	n and Remarks		
7.7.	7 77 T	Sample Recovery	ASTIV Froze	Frost ufc3-2	Blow (Symbol			%Gravel	%Sand	%Fines	% Finer 0.02mm	Max S) GIA	% Water					
- 57 57 57 57 57 57	7.74.77 7.74.77 7.74.77 7.74.77 7.74.77 7.74.78 7.74.78 7.74.74					PT- OL	Peat with Organic	Silt								Dark brown org	anics a	nd roots		
- 4		1		F4*	3 4 6	ML	SILT with Sand		9	10	81		1		26	Grey, mottled b nonplastic (NP)	ey, mottled brown, wet, angular gravel, fin nplastic (NP) fines			
- 6 ·		2		F2	6 11 25	SM	Silty SAND with Gr	ravel	28	48	24	14.7	1		27	Brown and black, wet, angular gravel, fine to m sand, NP fines, residual basalt Hard drilling at 6.5 feet				
- 8 - -10		34.			50/3	Rock	Rock									Weathered blac	k hasal	ıt		
- -12 -					inches	, Trock	rook									Bottom of Hole Elevation 26	10.3 ft. 9.1 ft.	Ionization Detector		
-14 -14																				
EXPLORATION LOG NEWTOK. GPJ ACE_ANC. GDT 7/1/09																				
TION LOG NEWT																				
* Indic	cates	Esti	mated	l Fros	t Classifi	cation	l		F	Proje	ct:Mei	tarvik	Town	site	<u> </u>	<u> </u>		Hole Number: AP-33		

((CORPS	OF E	ISTRICT ENGINEERS	Project			ik To	wnsite ska	9					Pag Date	e 1 of 1 e: 21 Aug 2008	
							g services Section	Drilling	Ager Other	-		☐ Ala rilling	ska D	istrict		Datum	: Vertica Horiza			
	E	XF	PL(ЭR	ATI	O۱	I LOG	Locatio	n:	Nortl East		of Hole vation:	229.4 ft.							
	e Nun 3-34	nber,	Field:		Permane AP-34	ent:		Operat Kelli				tor: gory Car	Carpenter							
'			□ d	_	ole \square	Monit	oring Well □ Pi	_ ezomete		pth to	Gro	undwa	ater:			Depth 15.0	Drilled:		Total Depth: 16.5 ft.	
Hai	nmer 0 lbs			Split	Spoon I. 5 in .	Size and Type (Type o		nples: pe & Cathead		
	10103		083				Classification	2400		Gra	in Size						Desc		n and Remarks	
Depth (ft.)	Lithology	Sample	ASTM D 4083 Frozen	Frost Class. ufc3-250-01fa	Blow Count	Symbol	ASTM: D 2487 or D 2	2488	%Gravel	%Sand	%Fines	% Finer 0.02mm	Max Size (in.)	PID (ppm)	% Water	SURFAC Tun				
	で 示 な で で な な な な な な な な な な な な な な な	4			_	PT- OL	Peat with Organic S	Silt	3				_			Dark br	own orgai	nics a	nd roots	
- 2 - - 4		1		F4*	3 4 6	ML	SILT		0	4	96				27	Brown	- — — — with rust s	wet, nonplastic (NP) fines		
- 6		2		F3*	6 17 23	SM	Silty SAND with Gr	ravel	29	33	38		1		21	Brown, fines	Brown, wet, angular gravel, fine to co			
— 8 -																Encour drilling		bles a	it eight feet, able to continue	
—10 -		3		F2	24 30 15	SM	Silty SAND with Gr	ravel	40	44	16	9.6	1		15	Brown coarse	and black sand, NP	, mois fines,	t, angular gravel, fine to residual basalt	
—12 -								. — — -								Very ha	ard drilling - — — —	j at 12 — —	feet	
		4		F4*	6 10 13	ML	Sandy SILT						0.5						wet, angular gravel, fine to residual basalt	
EXPLORATION LOG NEWTOK.GPJ ACE_ANC.GDT 7/1/09 *	erkel	- Proposition of														Eleva	of Hole 10 ation 212. Cold/Hot) I	9 ft.	Ionization Detector	
× * N	dicate	s Esti	mated	d Frost	t Classifi	cation			F	Projec	t:Mer	tarvik	Town	site					Hole Number:	

			OISTRICT ENGINEERS	Project		ertarv ewtok		wnsite ka)						e 1 of 1 e: 21 Aug 2008
LUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUUU			Drilling	-	-		☐ Alas	ska D	istrict		Datum	: Vertica Horizo		21 Aug 2008	
									op of Hole Elevation: 183.2 ft.						
Hole Number, Field: Permanent: TB-35 AP-35				Operator: I					Inspec	Inspector: Gregory Carpenter					
Type of Hole: ☐ (□ Moni	toring Well	 ezomete		epth to	Grou	undwa	nter:				Drilled:	•	Total Depth: 16.5 ft.
Hammer Weight: 340 lbs	Split Spoon 2.5 in.		Size and Type 8 in. HSA			-		of Equ			l		Type of Drive		nples: be & Cathead
.)	ss. -01fa int		Classification ASTM: D 2487 or D	2488		Gra	in Size	: 	(in.)	(-		SURFAC		riptior	and Remarks
Depth (ft.) Sample Recovery ASTM D 4083	Frost Class. ufc3-250-01fa Blow Count	Symbol		_ 100	%Gravel	%Sand	%Fines	% Finer 0.02mm	Max Size (in.)	PID (ppm)	% Water	Tun			
######################################		PT- OL	Peat with Organic S	Silt								Dark br	own orgar	nics ar	nd roots
- 4	F2 6 8 9	GM	Silty GRAVEL with	Sand	53	20	27	18.5	1		16	sand, n	and black, conplastic ((NP) fi	
- 6 <u>2</u>	F3* 10 11 24	SM	Silty SAND with Gr	ravel	27	41	32		1		18	Brown sand, N	and black, IP fines	wet, a	angular gravel, fine to coars
-10 1 3 3 -12 1 1 3 3 -12 1 1 3 3 3 3 3 3 3 3	F1 7 7 7	GM	Silty GRAVEL with	Sand	50	36	14	8.0	1		29				t, angular gravel, fine to residual basalt
- 14 1/108	F4* 6 8 8	ML	SILT with Sand						1			Brown		moist	eet i, angular gravel, fine to residual basalt
* Indicates Estimate												Eleva	of Hole 16 ation 166.7 Cold/Hot) F	7 ft.	Ionization Detector
* Indicates Estimate	d Frost Class	sification			 	l Projec	t:Mer	tarvik	Town	site					Hole Number: AP-35

APPENDIX ADDENDUM

Grain-size distribution curves

U.S. ARMY CORPS OF ENGINEERS SOILS AND GEOLOGY SECTION, ALASKA DISTRICT

Newtok Relocation Newtok. Alaska

	The state of the s	Unified Soil Classification	
	Frost	Class.	
	assing Passing	#200 0.02mm Class.	(%)
コロハロコ	Passing	#200	(%)
Š,	e;	·	Silt
ני	article Size	Analysis	
	Par	∢.	Gravel Sand
	imits	☲	
	erberg L	굽	
	₹	Ⅎ	
	Moisture	Bottom Content	(%)
	epth interval	Bottom	
	Depth	Top	
	,	Sample	Number
	Permanent	₽.	(Field)

(ML) Sitt with sand	(ML) Sift with sand	(SM) sifty sand	(ML) Sandy sift with gravel	(SM) Silty sand with gravel	(SM) Silty sand with gravel	(ML) Sandy silt	(ML) Silt	(ML) Gravelly silt	(ML) Sift	(ML) Silt	(ML) Silt	(ML) Gravelly silt with sand	(GM) Silty gravel with sand	(ML) Silt with sand	(ML) Silt with sand	(SM) Silty sand with gravel	(ML) Silt	(SM) Silty sand with gravel	(SM) Silty sand with gravel	(GM) Silty gravel with sand	(SM) Silty sand with gravel	(GM) Sifty gravel with sand
		F4			F3		F4			F4			F2			F2			F2	F2		ī
		26.5			15.7		63.4			48.1			18.4			14.7			9.6	18.5		8.0
72.3	75.3	47.4	63.7	41.3	35.6	69.0	87.1	60.7	98.2	98.3	95.1	50.1	28.3	73.4	9.08	24.5	96.2	38.0	16.3	27.3	32.2	13.8
20.1	14.5	42.4	19.9	38.7	48.8	21.6	7.0	11.9	1.8	1.7	3.5	22.6	30.5	18.2	10.1	47.7	3.8	33,2	44.2	19.8	41.3	36.0
7.6	10.2	10.3	16.4	20.0	15.6	9.4	5.9	27.4	0.0	0.0	1.4	27.3	41.2	8.4	9.3	27.8	0.0	28.8	39.5	52.9	26.5	50.2
	•			NP	NP									ΜÞ	ΝΡ							
35.4	29.3	26.9	42.6	43.1	23.5	26.1	27.6	23.3	27.6	30.6	25.8	29.8	25.7	35.3	26.2	27.1	27.0	21.3	15.4	16.0	18.4	28.7
4.0	4.0	6.0	4.0	6.0	16.0	4.0	4.0	6.0	4.0	0.9	11.0	4.0	0.0	11.0	4.0	0.9	4.0	0.9	11.0	4.0	0.9	11.0
2.5	2.5	4.5	2.5	4.5	14.5	2.5	2.5	4.5	2.5	4.5	9.5	2.5	4.5	9.5	2.5	4.5	2.5	4.5	9.5	2.5	4.5	9.5
۲-	τ-	2	-	2	3	-	_	7	τ-	2	ဗ	-	7	3	-	7	τ-	2	က	-	2	ဗ
TB-25	TB-26	TB-26	TB-27	TB-27	TB-27	TB-28	TB-29	TB-29	TB-30	TB-30	TB-30	TB-31	TB-31	TB-31	TB-33	TB-33	TB-34	TB-34	TB-34	TB-35	TB-35	TB-35

TERRA FIRMA TESTING

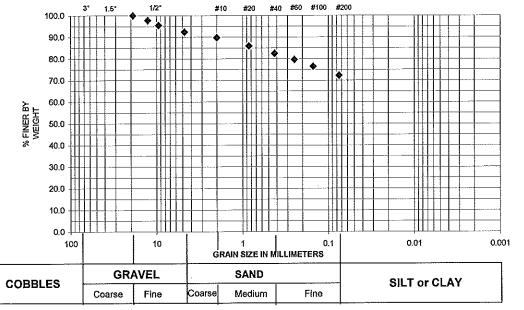
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO .: 2076-08 SAMPLE LOCATION: TB-25 (Depth 2.5' - 4.0') SAMPLE NO/ DEPTH DESCRIPTION: Silt with sand 10/17/2008 DATE TESTED: TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: 7.6	USC: ML
% SAND: 20.1	FC:
% SILT/CLAY: 72.3	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	35.4

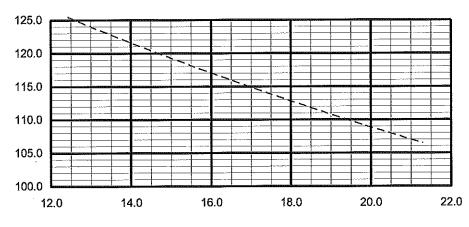
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

012121113121010112221								
SIEVE	SIEVE	TOTAL %						
SIZE (mm)	SIZE (în.)	PASSING	SPEC					
152.4	6ª							
76.2	3"							
38.1	1.5"							
19.05	3/4"	100						
12.7	1/2"	98						
9.5	3/8"	95						
4.75	# 4	92						
2	#10	90						
0.85	#20	86						
0.425	#40	82						
0.25	# 60	80						
0.15	#100	76						
0.075	#200	72.3						

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

	ELAPSED	DIAMETER	TOTAL %
	TIME	(mm)	PASSING
	0		
	0.5		
	1		
	2		
	4		
	8		
	15		
	30		
	60		
1	250		
	1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993

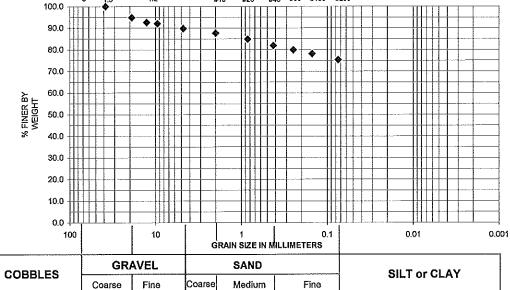
www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-26
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Silt with sand
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: _	10.2	USC:	ML
% SAND:	14.5	FC:	
% SILT/CLAY:	75.3	.02 mm:	
ASTM D1557(unco	rrected)		pcf
ASTM D1557(uncon ASTM D4718 (con			pcf pcf
	rrected)		

PARTICLE SIZE ANALYSIS ASTM D422/ C136

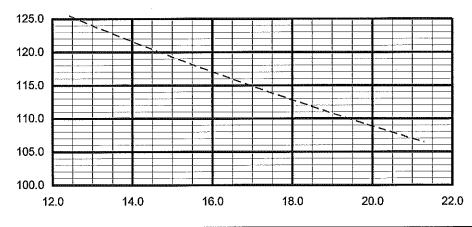
#40 #60 #100



SIEV	SIEVE ANALYSIS RESULT									
SIEVE	SIEVE	TOTAL %								
SIZE (mm)	SIZE (in.)	PASSING	SPEC							
152.4	6"									
76.2	3"									
38.1	1.5"	100								
19.05	3/4"	95								
12.7	1/2"	93								
9.5	3/8"	92								
4.75	#4	90								
2	#10	88								
0.85	#20	85								
0.425	#40	82								
0.25	#60	80								
0.15	#100	7 8								
0.075	#200	75.3								

Coarse Medium

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

HYDROMETER RESULT

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

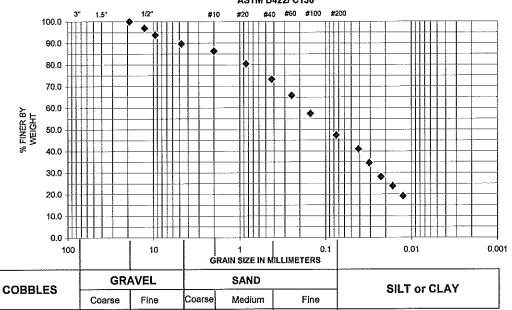
Telephone: (907) 344-5934 Fax: (907) 344-5993

x: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-26
SAMPLE NO/ DEPTH	SA-2 (Depth 4.5' - 6.0')
DESCRIPTION:	Silty sand.
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

ODAVEL.	40.0	USC:	eu.
% GRAVEL:_	10.3	080:_	SM
% SAND:	42.4	FC:_	F4
% SILT/CLAY:	47.4	.02 mm:	26.5
ASTM D1557(uncor			ocf
ASTM D4718 (corrected)			ocf
OPTIMUM M.C.% (corrected)			
NATURAL M.C. %		26.9	

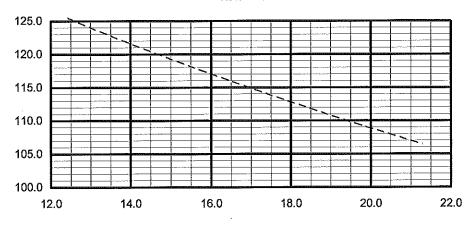
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	OIC IC		
	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	97	
9.5	3/8"	94	
4.75	#4	90	
2	#10	86	
0.85	#20	80	
0.425	#40	73	
0.25	#60	66	
0.15	#100	57	
0.075	#200	47.4	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0412	41.0
2	0.0309	34.6
4	0.0226	28.2
8	0.0166	23.9
15	0.0125	19.2
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

PROJECT CLIENT:

SAMPLE LOCATION:

SAMPLE NO/ DEPTH

Newtok Relocation

SA-1 (Depth 2.5' - 4.0')

Sandy silt with gravel

Ron Caron C.E.T.

2076-08

10/17/2008

TB-27

DP

PROJECT NAME:

PROJECT NO.:

DESCRIPTION:

DATE TESTED:

REVIEWED BY:

TESTED BY:

TERRA FIRMA TESTING

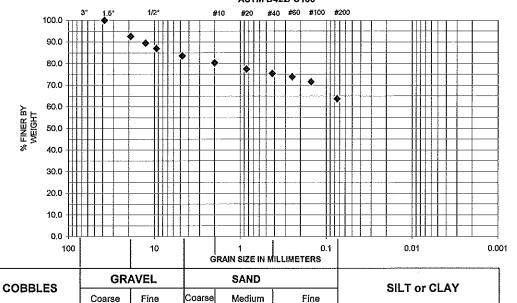
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

Corps of Engrs - Alaska District

% GRAVEL:16.4	USC: ML
% SAND: 19.9	FC:
% SILT/CLAY: 63.7	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	42.6

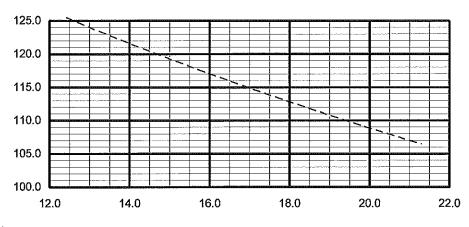
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	93	
12.7	1/2"	90	
9.5	3/8"	87	
4.75	# 4	84	
2	#10	80	
0.85	#20	77	
0.425	#40	75	
0.25	# 60	74	
0.15	#100	72	
0.075	#200	63.7	

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

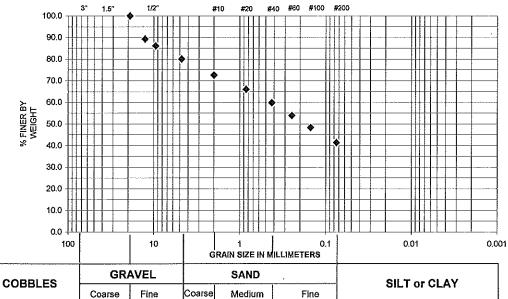
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-27
SAMPLE NO/ DEPTH	SA-2 (Depth 4.5' - 6.0')
DESCRIPTION:	Silty sand w/ gravel
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL:	USC: SM
% SAND: 38.7	FC:
% SILT/CLAY: 41.3	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	43.1

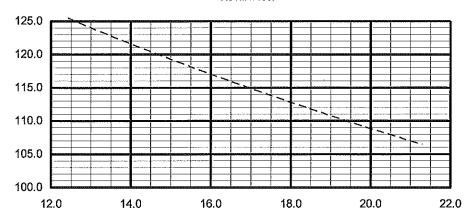
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	89	
9.5	3/8"	86	
4.75	#4	80	
2	#10	73	
0.85	#20	66	
0.425	#4 0	60	
0.25	# 60	54	
0.15	#100	48	
0.075	#200	41.3	

SILT or CLAY Fine Coarse Medium Fine

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

HYDROMETER RESULT

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	Non
ASTM 4318	Piastic

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

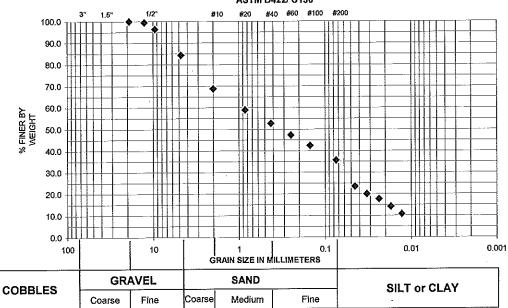
Telephone: (907) 344-5934 Fax: (907) 344-5993

x: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-27
SAMPLE NO/ DEPTH	SA-3 (Depth 14.5' - 16.0')
DESCRIPTION:	Silty sand w/ gravel
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL:15.6	USC:_	SM
% SAND: 48.8	FC:_	F3
% SILT/CLAY: 35.6	.02 mm:	15.7
ASTM D1557(uncorrected)		ocf
ASTM D4718 (corrected)		ocf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	23.5	

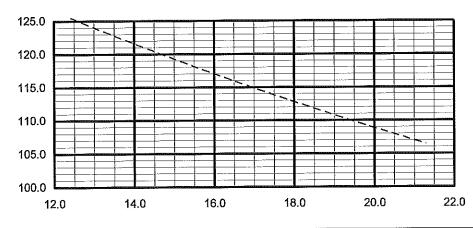
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

O/D/ PA / ((10 / 10 / 10 / 10 / 10 / 10 / 10 /			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	99	
9.5	3/8"	96	
4.75	# 4	84	
2	#10	69	
0.85	#20	59	
0.425	#40	53	
0.25	#60	47	
0.15	#100	42	
0.075	#200	35.6	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0452	23.5
2	0.0328	20.1
4	0.0237	17.6
8	0.0173	14.2
15	0.0129	10.7
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	Non
ASTM 4318	Plastic

REVIEWED BY:

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

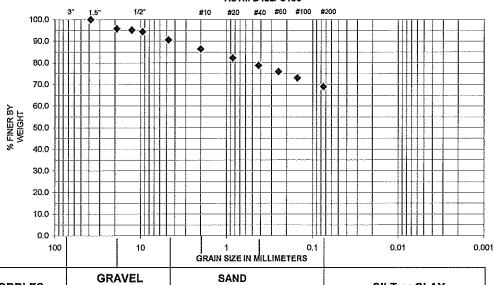
Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 SAMPLE LOCATION: TB-28 SAMPLE NO/ DEPTH SA-1 (Depth 2.5' - 4.0') DESCRIPTION: Sandy sllt DATE TESTED: 10/17/2008 TESTED BY: DP

Ron Caron C.E.T.

% GRAVEL:	9.4	USC: ML
% SAND:	21.6	FC:
% SILT/CLAY:	69.0	.02 mm:
ASTM D1557(uncon	rected)	pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %		26.1

PARTICLE SIZE ANALYSIS ASTM D422/ C136

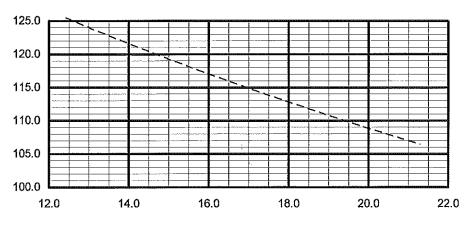


	SIEVE	SIEVE	TOTAL %	
	SIZE (mm)	SIZE (in.)	PASSING	SPEC
	152.4	6"		
	76.2	3"		
-	38.1	1.5"	100	
	19.05	3/4"	96	
ĺ	12.7	1/2"	95	
	9.5	3/8"	94	
	4.75	# 4	91	
	2	#10	86	
	0.85	#20	82	
	0.425	#40	79	
	0.25	#60	76	
	0.15	#100	73	

SIEVE ANALYSIS RESULT

COBBLES SILT or CLAY Coarse Fine Coarse Medium Fine

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		_
1440		

0.075 #200 **69.0**

HYDROMETER RESULT

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

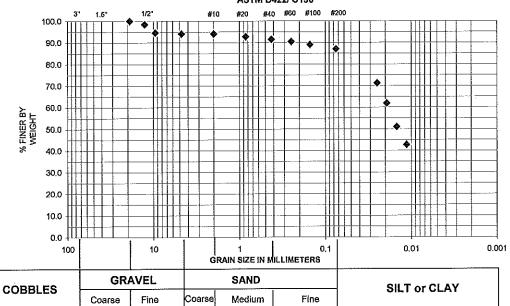
Telephone: (907) 344-5934 Fax: (907) 344-5993

www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-29
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Silt
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

	· · · · · · · · · · · · · · · · · · ·	
% GRAVEL: 5.9	USC:_	ML
% SAND: 7.0	FC:_	F4
% SILT/CLAY: 87.1	.02 mm:	63.4
ASTM D1557(uncorrected)		ocf
ASTM D4718 (corrected)		ocf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	27.6	•

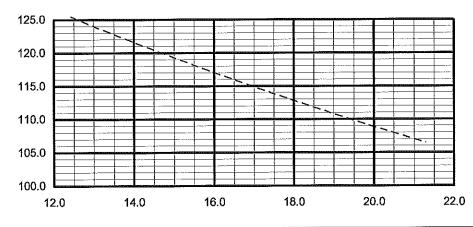
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (în.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	99	
9.5	3/8"	95	
4.75	# 4	94	
2	#10	94	
0.85	#20	93	
0.425	#40	92	
0.25	#60	90	
0.15	#100	89	
0.075	#200	87.1	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

III DI TOME I ENTRE NECOTAL			
ELAPSED	DIAMETER	TOTAL %	
TIME	(mm)	PASSING	
0			
0.5			
1			
2	0.0249	71.3	
4	0.0193	61.9	
8	0.0148	51.0	
15	0.0113	42.7	
30			
60			
250			
1440			

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

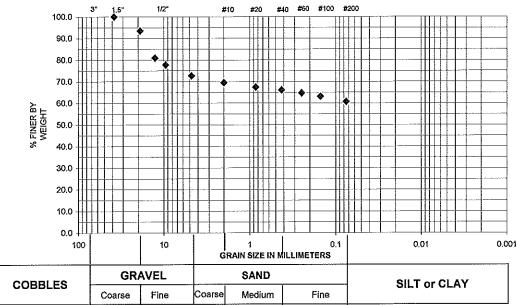
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-29
SAMPLE NO/ DEPTH	SA-2 (Depth 4.5' - 6.0')
DESCRIPTION:	Gravelly silt
DATE TESTED:	10/17/2008
TESTED BY:	DP
DEVIEWED DV:	Pon Caron C E T

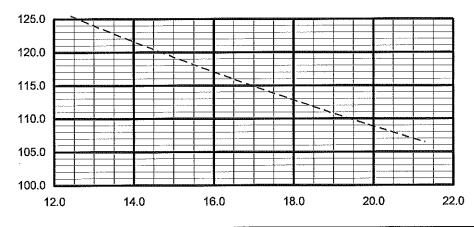
% GRAVEL:	27.4	USC:	ML
% SAND:	11.9	FC:	
% SILT/CLAY:	60.7	.02 mm:	
ASTM D1557(uncom	ected)		pcf
ASTM D4718 (corre	ected)		pcf
ODTIVUUL O O C	vrected)		
OPTIMUM M.C.% (co	nicology		

PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT				
SIEVE	SIEVE	TOTAL %		
SIZE (mm)	SIZE (in.)	PASSING	SPEC	
152.4	6"			
76.2	3"			
38.1	1.5"	100		
19.05	3/4"	93		
12.7	1/2"	81		
9.5	3/8"	78		
4.75	#4	73		
2	#10	69		
0.85	#20	67		
0.425	#40	66		
0.25	# 60	65		
0.15	#100	63		
0.075	#200	60.7		

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

HYDROMETER RESULT

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

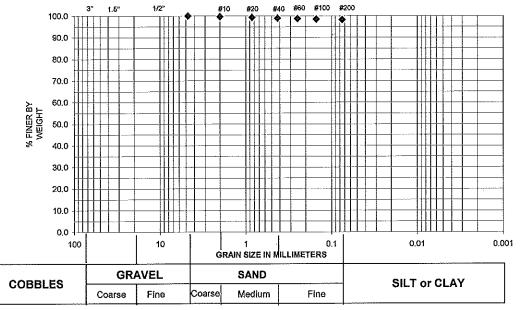
Telephone: (907) 344-5934

Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-30
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Silt
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

NATURAL M.C. %	27.6
OPTIMUM M.C.% (corrected)	
ASTM D4718 (corrected)	pcf
ASTM D1557(uncorrected)	pcf
% SILT/CLAY: 98.2	.02 mm:
% SAND: 1.8	FC:
% GRAVEL: 0.0	USC: ML

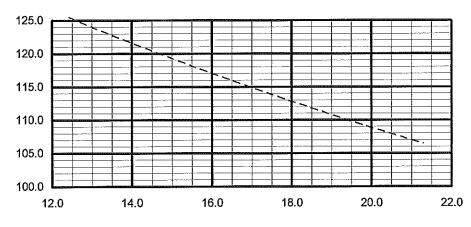
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"		
9.5	3/8"		
4.75	#4	100	
2	#10	100	
0.85	#20	99	
0.425	#40	99	
0.25	# 60	99	
0.15	#100	99	
0.075	#200	98.2	

MOISTURE-DENSITY RELATIONSHIP **ASTM D1657**



HYDROMETER RESULT

ELAPS	ED D	IAMETER	TOTAL %
TIME	:	(mm)	PASSING
0			
0.5			
1			
2			
4			
8			
15			
30			
60			
250)		
144	0		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

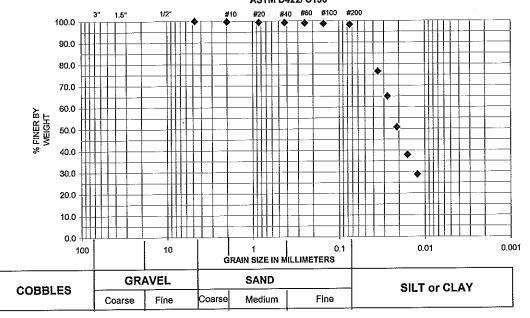
Telephone: (907) 344-5934 Fax: (907) 344-5993

www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-30
SAMPLE NO/ DEPTH	SA-2 (Depth 4.5' - 6.0')
DESCRIPTION:	Silt
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 0.0	USC:_	ML
% SAND: 1.7	FC:	F4
% SILT/CLAY: 98.3	.02 mm:	48.1
ASTM D1557(uncorrected)		pcf
ASTM D4718 (corrected)		pcf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	30.6	

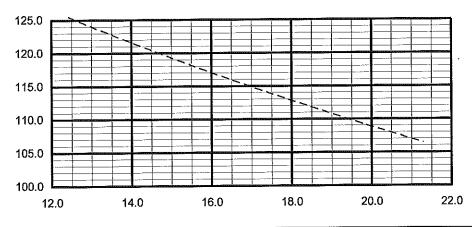
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

ŞIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"		
9.5	3/8"		****
4.75	# 4	100	
2	#10	100	
0.85	#20	99	
0.425	#40	99	
0.25	# 60	99	
0.15	#100	99	
0.075	#200	98.3	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0352	76.6
2	0.0273	65.1
4	0.0212	50.7
8	0.0160	38.0
15	0.0123	28.8
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

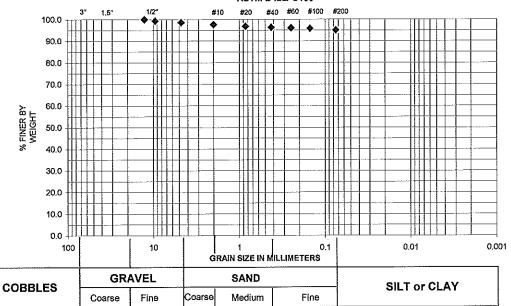
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO .: 2076-08 SAMPLE LOCATION: TB-30 SAMPLE NO/ DEPTH SA-3 (Depth 9.5' - 11.0') DESCRIPTION: Silt 10/17/2008 DATE TESTED: TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: 1.4 % SAND: 3.5 % SILT/CLAY: 95.1	USC: ML FC: .02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	25.8

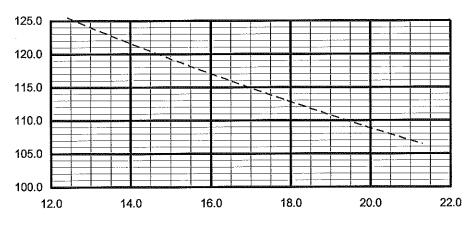
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"	100	
9.5	3/8"	99	
4.75	# 4	99	
2	#10	98	
0.85	#20	97	
0.425	#40	96	
0.25	# 60	96	
0.15	#100	96	
0.075	#200	95.1	
0.075	#200	95.1	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

	ELAPSED	DIAMETER	TOTAL %
L	TIME	(mm)	PASSING
	0		
	0.5		
	1		
L	2		
	4		
E	8		
	15		
Ι	30		
I	60		
ſ	250		
ſ	1440		
_	•		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

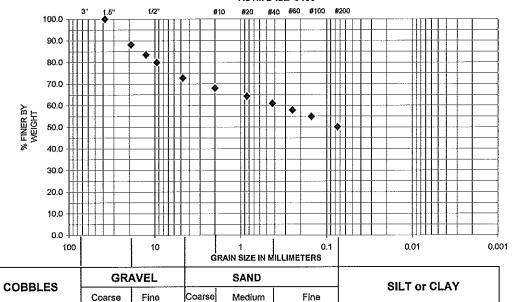
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-31
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Gravelly silt with sand
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 27.3	USC: ML	
% SAND: 22.6	FC:	
% SILT/CLAY: 50.1	.02 mm:	
ASTM D1557(uncorrected)	pcf	
ASTM D4718 (corrected)		
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	29.8	

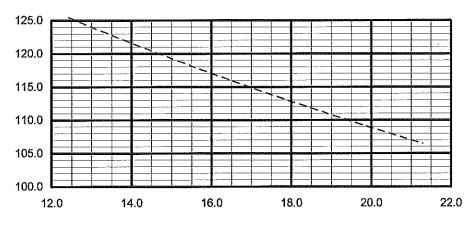
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEV	E ANALY	SIS RI	<u>ESULT</u>
SIE//E	SIEV/E	TOTAL	0/.

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	88	
12.7	1/2"	83	
9.5	3/8"	80	
4.75	# 4	73	
2	#10	68	
0.85	#20	64	
0.425	#40	61	
0.25	# 60	58	
0.15	#100	55	
0.075	#200	50.1	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

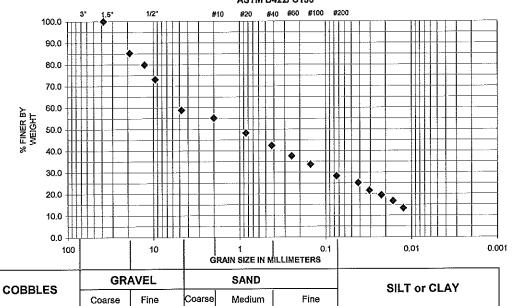
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 TB-31 SAMPLE LOCATION: SAMPLE NO/ DEPTH SA-2 (Depth 4.5' - 6.0') DESCRIPTION: Silty gravel w/ sand 10/17/2008 DATE TESTED: TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: 41.2		USC:_	GM
% SAND: 30.5		FC:	F2
% SILT/CLAY: 28.3		.02 mm:	18.4
ASTM D1557(uncorrected)			pcf
ASTM D4718 (corrected)			ocf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %		25.7	

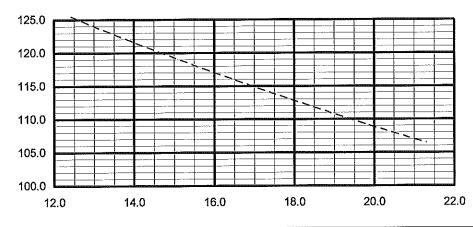
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	85	
12.7	1/2"	80	
9.5	3/8"	73	
4.75	#4	59	
2	#10	55	
0.85	#20	48	
0.425	#40	42	
0.25	# 60	38	
0.15	#100	34	
0.075	#200	28.3	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0419	25.2
2	0.0309	21.6
4	0.0224	19.4
8	0.0164	16.7
15	0.0124	13.4
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

REVIEWED BY:

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

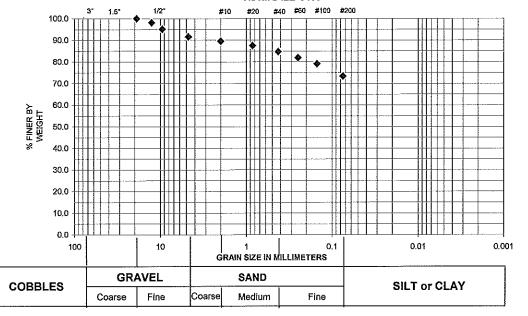
Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 SAMPLE LOCATION: TB-31 SA-3 (Depth 9.5' - 11.0') SAMPLE NO/ DEPTH DESCRIPTION: Silt with sand 10/17/2008 DATE TESTED: TESTED BY: DP

Ron Caron C.E.T.

% GRAVEL: 8.4	USC: ML
% SAND: 18.2	FC:
% SILT/CLAY: 73.4	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected) pcf	
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	35.3

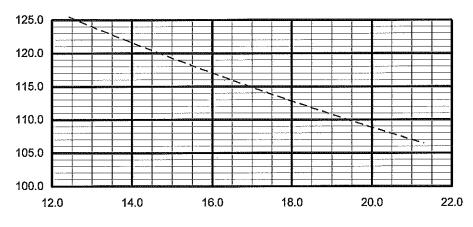
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (în.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"	100	
12.7	1/2"	98	
9.5	3/8"	95	
4.75	#4	92	
2	#10	90	
0.85	#20	87	
0.425	#40	85	
0.25	#60	82	
0.15	#100	79	
0.075	#200	73.4	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	Non
ASTM 4318	Plastic

TERRA FIRMA TESTING

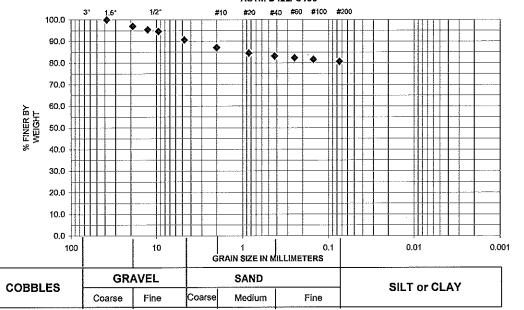
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 SAMPLE LOCATION: TB-33 SA-1 (Depth 2.5' - 4.0') SAMPLE NO/ DEPTH DESCRIPTION: Silt with sand 10/17/2008 DATE TESTED: TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: 9.3	USC: ML
% SAND: 10.1	FC:
% SILT/CLAY: 80.6	.02 mm:
	· · · · · · · · · · · · · · · · · · ·
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	26.2

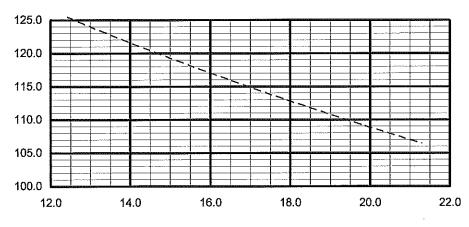
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	97	
12.7	1/2"	95	
9.5	3/8"	95	
4.75	#4	91	
2	#10	87	
0.85	#20	85	
0.425	#40	83	
0.25	# 60	82	
0.15	#100	82	
0.075	#200	80.6	
0.075	#200	80.6	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	Non
ASTM 4318	Plastic

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

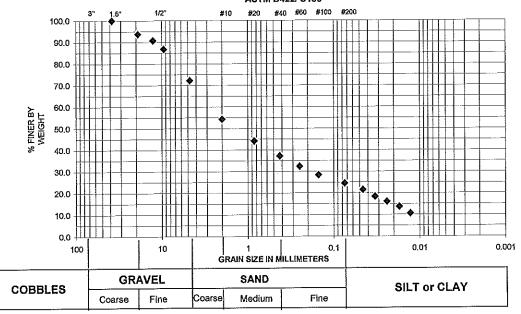
Telephone: (907) 344-5934 Fax: (907) 344-5993

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PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-33
SAMPLE NO/ DEPTH	SA-2 (Depth 4.5' - 6.0')
DESCRIPTION:	Silty sand w/ gravel
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 27.8	USC: SM
% SAND: 47.7	FC: F2
% SILT/CLAY: 24.5	.02 mm: 14.7
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	27.1

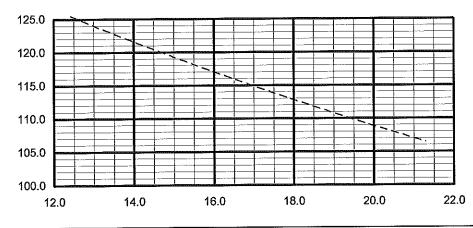
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	94	
12.7	1/2"	91	
9.5	3/8"	87	
4.75	# 4	72	
2	#10	54	
0.85	#20	44	
0.425	#40	37	
0.25	# 60	32	
0.15	#100	28	
0.075	#200	24.5	

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0458	21.5
2	0.0331	18.4
4	0.0240	16.0
8	0.0173	13.7
15	0.0129	10.6
30		
60		
250		l .
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

REVIEWED BY:

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

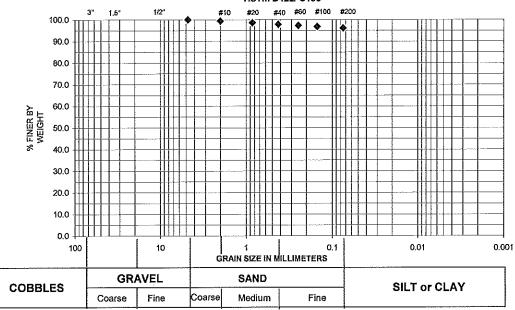
Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-34
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Silt
DATE TESTED:	10/17/2008
TESTED BY:	DP

Ron Caron C.E.T.

% GRAVEL: 0.0	USC: ML
% SAND: 3.8	FC:
% SILT/CLAY: 96.2	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	27.0

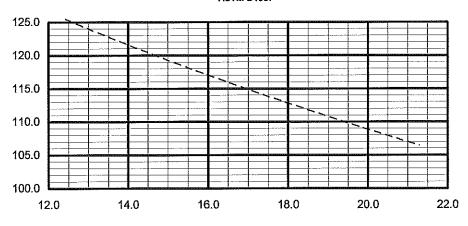
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"		
19.05	3/4"		
12.7	1/2"		
9.5	3/8"		
4.75	#4	100	
2	#10	99	
0.85	#20	99	
0.425	#40	98	
0.25	#60	97	
0.15	#100	97	
0.075	#200	96.2	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30.		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

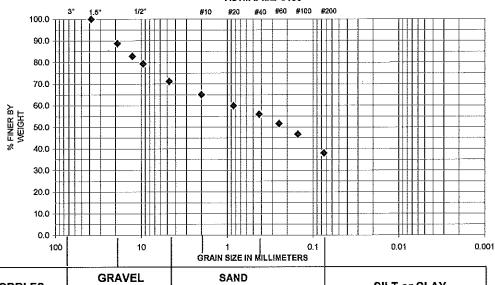
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 SAMPLE LOCATION: TB-34 SAMPLE NO/ DEPTH SA-2 (Depth 4.5' - 6.0') DESCRIPTION: Silty sand w/ gravel DATE TESTED: 10/17/2008 TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% SAND: 33.2	FC:
% SILT/CLAY: 38.0	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	21.3

PARTICLE SIZE ANALYSIS ASTM D422/ C136



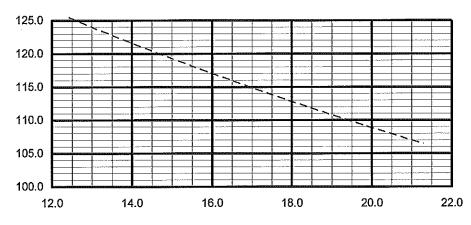
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	·
19.05	3/4"	89	
12.7	1/2"	83	
9.5	3/8"	79	
4.75	# 4	71	
2	#10	65	
0.85	#20	60	
0.425	#40	56	
0.25	# 60	52	
0.15	#100	47	

SIEVE ANALYSIS RESULT

COBBLES GRAVEL SAND

Coarse Fine Coarse Medium Fine SILT or CLAY

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



	METER F	
ELAPSED	DIAMETER	TOTAL 9

#200

38.0

0.075

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1		
2		
4		
8		
15		
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

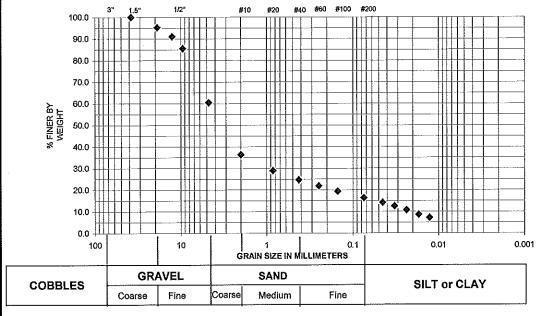
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-34
SAMPLE NO/ DEPTH	SA-3 (Depth 9.5' - 11.0')
DESCRIPTION:	Silty sand w/ gravel
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: 39.5 % SAND: 44.2 % SILT/CLAY: 16.3	USC: SM FC: F2 .02 mm: 9.6
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	15.4

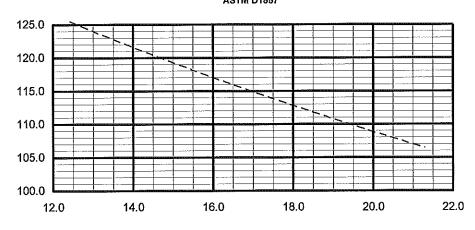
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	95	
12.7	1/2"	91	
9.5	3/8"	86	
4.75	#4	60	
2	#10	36	
0.85	#20	29	
0.425	#40	25	
0.25	# 60	22	
0.15	#100	19	

16.3

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT				
ELAPSED	DIAMETER	TOTAL %		
TIME	(mm)	PASSING		
0				
0.5				
1	0.0452	14.2		
2	0.0328	12.5		
4	0.0237	10.7		
8	0.0171	8.6		
15	0.0128	7.2		
30				
60				
250				
1440				

#200

0.075

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

Laboratory Testing / Construction Monitoring

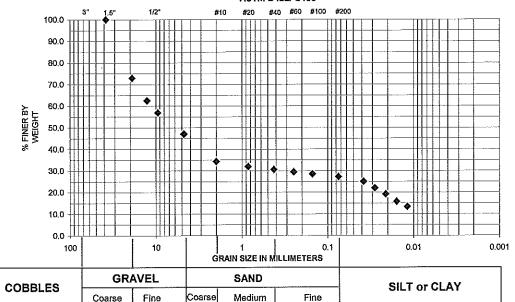
Telephone: (907) 344-5934 Fax: (907) 344-5993

www.nge-tft.com

PROJECT CLIENT:	Corps of Engrs - Alaska District
PROJECT NAME:	Newtok Relocation
PROJECT NO.:	2076-08
SAMPLE LOCATION:	TB-35
SAMPLE NO/ DEPTH	SA-1 (Depth 2.5' - 4.0')
DESCRIPTION:	Silty gravel w/ sand
DATE TESTED:	10/17/2008
TESTED BY:	DP
REVIEWED BY:	Ron Caron C.E.T.

% GRAVEL: <u>52.9</u>	USC: GM
% SAND: 19.8	FC: F2
% SILT/CLAY: 27.3	.02 mm: 18.5
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	16.0

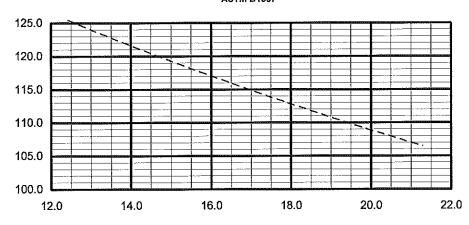
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT

SILVE MITALI DIO NEGOLI			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	73	
12.7	1/2"	63	
9.5	3/8"	57	
4.75	#4	47	
2	#10	34	
0.85	#20	32	
0.425	#40	31	
0.25	# 60	30	
0.15	#100	29	
0.075	#200	27.3	

MOISTURE-DENSITY RELATIONSHIP ASTM D1557



HYDROMETER RESULT

ITI DIVONILI EN NEGOLI			
ELAPSED	DIAMETER	TOTAL %	
TIME	(mm)	PASSING	
0			
0.5			
1	0.0381	25.0	
2	0.0283	22.0	
4	0.0212	19.2	
8	0.0158	15.8	
15	0.0118	13.4	
30			
60			
250			
1440			

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

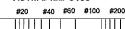
Laboratory Testing / Construction Monitoring

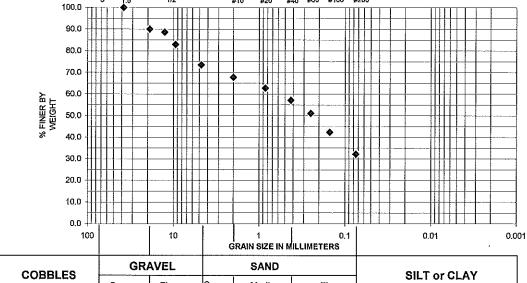
Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO.: 2076-08 SAMPLE LOCATION: TB-35 SAMPLE NO/ DEPTH SA-2 (Depth 4.5' - 6.0') DESCRIPTION: Silty sand wi gravel DATE TESTED: 10/17/2008 TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: <u>26.5</u>	USC: SM
% SAND: 41.3	FC:
% SILT/CLAY: 32.2	.02 mm:
ASTM D1557(uncorrected)	pcf
ASTM D4718 (corrected)	pcf
OPTIMUM M.C.% (corrected)	
NATURAL M.C. %	18.4

PARTICLE SIZE ANALYSIS ASTM D422/ C136

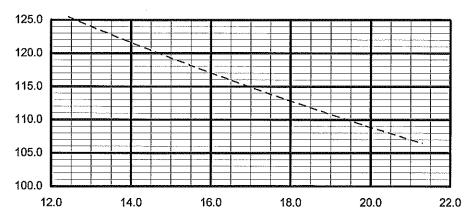




SIEVE ANALYSIS RESULT				
SIEVE	SIEVE	TOTAL %		
SIZE (mm)	SIZE (in.)	PASSING	SPEC	
152.4	6"			
76.2	3"		•	
38.1	1.5"	100		
19.05	3/4"	90		
12.7	1/2"	89		
 9.5	3/8"	83		
 4.75	#4	73		
2	#10	68		
0.85	#20	63		
0.425	#40	57		
0.25	# 60	51		
0.15	#100	42		
0.075	#200	32.2		

Coarse Fine Coarse Medium Fine

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT				
ELAPSED	DIAMETER	TOTAL %		
TIME	(mm)	PASSING		
0				
0.5				
1				
2				
4				
8				
15				
30				
60				
250				
1440				

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	

TERRA FIRMA TESTING

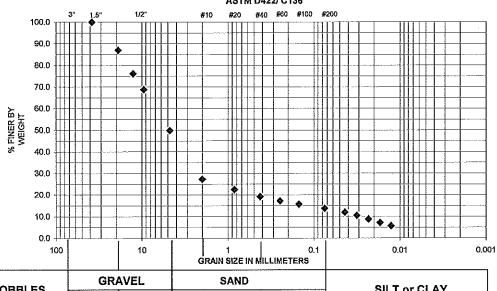
Laboratory Testing / Construction Monitoring

Telephone: (907) 344-5934 Fax: (907) 344-5993 www.nge-tft.com

PROJECT CLIENT: Corps of Engrs - Alaska District PROJECT NAME: **Newtok Relocation** PROJECT NO .: 2076-08 SAMPLE LOCATION: TB-35 SAMPLE NO/ DEPTH SA-3 (Depth 9.5' - 11.0') DESCRIPTION: Silty gravel w/ sand DATE TESTED: 10/17/2008 TESTED BY: DP REVIEWED BY: Ron Caron C.E.T.

% GRAVEL: 50.2	USC:	GM
% SAND: 36.0	FC:	F1
% SILT/CLAY: 13.8	.02 mm:	8.0
ASTM D1557(uncorrected)		ocf
ASTM D4718 (corrected)		ocf
OPTIMUM M.C.% (corrected)		
NATURAL M.C. %	28.7	

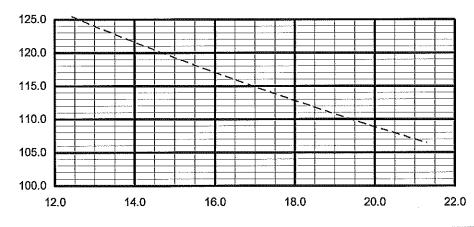
PARTICLE SIZE ANALYSIS ASTM D422/ C136



SIEVE ANALYSIS RESULT			
SIEVE	SIEVE	TOTAL %	
SIZE (mm)	SIZE (in.)	PASSING	SPEC
152.4	6"		
76.2	3"		
38.1	1.5"	100	
19.05	3/4"	87	
12.7	1/2"	76	
9.5	3/8"	69	
4.75	#4	50	
2	#10	27	
0.85	#20	22	
0.425	#40	19	
0.25	# 60	17	
0.15	#100	16	
0.075	#200	13.8	

COBBLES SILT or CLAY Coarse Coarse Fine Medium Fine

MOISTURE-DENSITY RELATIONSHIP **ASTM D1557**



HYDROMETER RESULT

ELAPSED	DIAMETER	TOTAL %
TIME	(mm)	PASSING
0		
0.5		
1	0.0437	12.0
2	0.0317	10.5
4	0.0232	8.8
8	0.0170	7.2
15	0.0127	5.7
30		
60		
250		
1440		

Perm.	
(ASTM D2438)	
Degradation	
(ATM T-13)	
Atterberg Limit	
ASTM 4318	